Attachment 1, Table G-3

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RSA-271, RCRA Facility Investigation
Redstone Arsenal, Madison County, Alabama

Parameter	Unit	Total Samples Collected	Total Samples Analyzed	Detections	Min Value	Max Value	Detections Above Criteria	BSV[a]	Min Value	Max Value	Detections Above 2021 Criteria	2014 PSV[b]	2021 PSV[b]	Min Value	Max Value	2016 PSV[c]
Chrysene[d]	µg/L	11	7	1	0.0842	0.0842	-	NA	-	-	-	25	25	-	-	25
Dibenz(a,h)anthracene[d]	µg/L	11	7	1	0.0885	0.0885	-	NA	0.0885	0.0885	1	0.025	0.025	0.0885	0.0885	0.025
Dibenzofuran	µg/L	11	7	2	2.05	2.67	-	NA	2.05	2.67	2	0.79	0.79	2.05	2.67	0.79
Diethyl phthalate	µg/L	11	7	0	-	-	-	NA	-	-	-	1500	1500	-	-	1500
Dimethyl phthalate	µg/L	11	7	1	5.2	5.2	-	NA	-	-	-	NA	NA	-	-	NA
Di-n-butyl phthalate	µg/L	11	7	0	-	-	-	NA	-	-	-	90	90	-	-	90
Di-n-octyl phthalate	µg/L	11	7	0	-	-	-	NA	-	-	-	20	20	-	-	20
Fluoranthene	µg/L	11	7	2	0.0557	0.0891	-	NA	-	-	-	80	80	-	-	80
Fluorene	µg/L	11	7	3	1.9	2.64	-	NA	-	-	-	29	29	-	-	29
Hexachlorobenzene	µg/L	11	7	0	-	-	-	NA	-	-	-	1	1	-	-	1
Hexachlorobutadiene	µg/L	11	7	0	-	-	-	NA	-	-	-	0.3	0.14	-	-	0.14
Hexachlorocyclopentadiene	µg/L	11	7	0	-	-	-	NA	-	-	-	50	50	-	-	50
Hexachloroethane	µg/L	11	7	0	-	-	-	NA	-	-	-	0.69	0.33	-	-	0.33
Indeno(1,2,3-cd)pyrene[d]	µg/L	11	7	1	0.0953	0.0953	-	NA	-	-	-	0.25	0.25	-	-	0.25
Isophorone	µg/L	11	7	0	-	-	-	NA	-	-	-	78	78	-	-	78
Naphthalene	µg/L	11	7	4	0.63	9.5	-	NA	0.63	9.5	4	0.17	0.12	0.63	9.5	0.17
Nitrobenzene	µg/L	11	7	0	-	-	-	NA	-	-	-	0.14	0.14	-	-	0.14
n-Nitroso-di-n-propylamine	µg/L	11	7	0	-	-	-	NA	-	-	-	0.011	0.011	-	-	0.011
n-Nitrosodiphenylamine	µg/L	11	7	0	-	-	-	NA	-	-	-	12	12	-	-	12
Pentachlorophenol	µg/L	11	7	0	-	-	-	NA	-	-	-	1	1	-	-	1
Phenanthrene	µg/L	11	7	4	1.3	2.29	-	NA	-	-	-	18	12	-	-	18
Phenol	µg/L	11	7	0	-	-	-	NA	-	-	-	580	580	-	-	580
Pyrene	µg/L	11	7	1	0.0778	0.0778	-	NA	-	-	-	12	12	-	-	12
VOLATILES																
1,1,1-Trichloroethane	µg/L	11	10	0	-	-	-	NA	-	-	-	200	200	-	-	200
1,1,2,2-Tetrachloroethane	µg/L	11	10	0	-	-	-	NA	-	-	-	0.076	0.076	-	-	0.076
1,1,2-Trichloroethane	µg/L	11	10	0	-	-	-	NA	-	-	-	5	5	-	-	5
1,1-Dichloroethane	µg/L	11	10	0	-	-	-	NA	-	-	-	2.7	2.8	-	-	2.8
1,1-Dichloroethene	µg/L	11	10	0	-	-	-	NA	-	-	-	7	7	-	-	7
1,2,4-Trichlorobenzene	µg/L	11	10	0	-	-	-	NA	-	-	-	70	70	-	-	70
1,2-Dichlorobenzene	µg/L	11	10	0	-	-	-	NA	-	-	-	600	600	-	-	600
1,2-Dichloroethane	µg/L	11	10	0	-	-	-	NA	-	-	-	5	5	-	-	5
1,2-Dichloropropane	µg/L	11	10	0	-	-	-	NA	-	-	-	5	5	-	-	5
1,3-Dichlorobenzene	µg/L	11	10	0	-	-	-	NA	-	-	-	NA	NA	-	-	NA
1,4-Dichlorobenzene	µg/L	11	10	0	-	-	-	NA	-	-	-	75	75	-	-	75
2-Butanone	µg/L	11	10	0	-	-	-	NA	-	-	-	560	560	-	-	560
2-Hexanone	µg/L	11	10	0	-	-	-	NA	-	-	-	3.8	3.8	-	-	3.8
4-Methyl-2-pentanone	µg/L	11	10	0	-	-	-	NA	-	-	-	120	630	-	-	630
Acetone	µg/L	11	10	2	45.3	162	-	NA	-	-	-	1400	1400	-	-	1400
Benzene	µg/L	11	10	0	-	-	-	NA	-	-	-	5	5	-	-	5
Bromodichloromethane	µg/L	11	10	0	-	-	-	NA	-	-	-	80	80	-	-	80
Bromoform	µg/L	11	10	0	-	-	-	NA	-	-	-	80	80	-	-	80
Bromomethane	µg/L	11	10	0	-	-	-	NA	-	-	-	0.75	0.75	-	-	0.75
Carbon disulfide	µg/L	11	10	0	-	-	-	NA	-	-	-	81	81	-	-	81
Carbon tetrachloride	µg/L	11	10	1	0.808	0.808	-	NA	-	-	-	5	5	-	-	5
Chlorobenzene	µg/L	11	10	3	1.8	9	-	NA	-	-	-	100	100	-	-	100
Chloroethane	µg/L	11	10	0	-	-	-	NA	-	-	-	2100	2100	-	-	2100
Chloroform	µg/L	11	10	7	0.48	1.66	-	NA	-	-	-	80	80	-	-	80
Chloromethane	µg/L	11	10	0	-	-	-	NA	-	-	-	19	19	-	-	19
cis-1,2-Dichloroethene	µg/L	11	10	1	2.3	2.3	-	NA	-	-	-	70	70	-	-	70
cis-1,3-Dichloropropene	µg/L	11	10	0	-	-	-	NA	-	-	-	0.47	0.47	-	-	0.47
Dibromochloromethane	µg/L	11	10	0	-	-	-	NA	-	-	-	80	80	-	-	80

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Dichlorodifluoromethane	µg/L	11	10	0	-	-	-	NA	-	-	-	20	20	-	-	20
Ethylbenzene	µg/L	11	10	2	0.595	0.74	-	NA	-	-	-	700	700	-	-	700
Freon 113	µg/L	11	10	4	0.6	26.2	-	NA	-	-	-	5500	1000	-	-	5500
Methyl tert-butyl ether	µg/L	11	10	0	-	-	-	NA	-	-	-	14	14	-	-	14
Methylene chloride	µg/L	11	10	0	-	-	-	NA	-	-	-	5	5	-	-	5
Styrene	µg/L	11	10	0	-	-	-	NA	-	-	-	100	100	-	-	100
Tetrachloroethene	µg/L	11	10	1	0.47	0.47	-	NA	-	-	-	5	5	-	-	5
Toluene	µg/L	11	10	2	0.783	4.94	-	NA	-	-	-	1000	1000	-	-	1000
trans-1,2-Dichloroethene	µg/L	11	10	0	-	-	-	NA	-	-	-	100	100	-	-	100
trans-1,3-Dichloropropene	µg/L	11	10	0	-	-	-	NA	-	-	-	0.47	0.47	-	-	0.47
Trichloroethene	µg/L	11	10	7	0.588	190	-	NA	-	-	3	5	5	28	190	5
Trichlorofluoromethane	µg/L	11	10	1	13	13	-	NA	-	-	-	110	520	-	-	520
Vinyl chloride	µg/L	11	10	0	-	-	-	NA	-	-	-	2	2	-	-	2
Xylene, Total	µg/L	11	10	0	-	-	-	NA	-	-	-	10000	10000	-	-	10000

The number of detections excludes data with a "UB" qualifier as UB-qualified results are considered nondetect.
"-" - Indicates the parameter was not detected.
µg/L - Micrograms per liter.
NA - Screening value not available; value not applicable; or not analyzed for parameter.
PAH - Polynuclear aromatic hydrocarbons.

[a] Background screening values (BSV) for metals, except where noted in Shaw (2003), are based on the 95 percent upper tolerance limit of the unfiltered groundwater background data set (Shaw, 2003, *Final Methodology for the Comparison of Site and Background Data*, *Redstone Arsenal, Madison County, Alabama,* Prepared for the U.S. Army Corps of Engineers, Savannah District, November).

[b] Unless otherwise indicated, the Redstone-specific preliminary screening levels (PSV) for groundwater are determined by the following hierarchy:
1) Maximum contaminant level (EPA, 2012, *2012 Edition of the Drinking Water Standards and Health Advisories*, EPA 822-S-12-001, Office of Water, Washington, District of Columbia, April).
2) Tap water regional screening levels (RSL), adjusted, if necessary to reflect an incremental lifetime cancer risk of 1E-6 or a hazard index of 0.1 (EPA, 2014, *Regional Screening Levels for Chemical Contaminants at Superfund Sites,* Mid-Atlantic Risk Assessment, May).
3) In the absence of a published RSL, the PSV developed from the RSL for a structurally similar surrogate may be provided.
The health advisory level (HAL) is used for perchlorate (DOD, 2009, *Perchlorate Release Management Policy*, Memorandum from Wayne Arny to Deputy Assistant Secretaries of the Army, Navy and Air Force, 4 April).
The pdf version of the 2014 May RSL table is provided on the compact disk for this RFI Report.

[c] Unless otherwise indicated, the Redstone-specific PSVs for groundwater are determined by the following hierarchy:
1) MCL (EPA, 2012, *2012 Edition of the Drinking Water Standards and Health Advisories*, EPA 822-S-12-001, Office of Water, Washington, District of Columbia, April).
2) Tap water regional screening levels (RSL), adjusted, if necessary to reflect an incremental lifetime cancer risk of 1E-6 or a hazard index of 0.1 (EPA, 2016, *Regional Screening Levels for Chemical Contaminants at Superfund Sites,* May).
3) In the absence of a published RSL, the PSV developed from the RSL for a structurally similar surrogate may be provided.
The HAL is used for perchlorate (DOD, 2009, *Perchlorate Release Management Policy*, Memorandum from Wayne Arny to Deputy Assistant Secretaries of the Army, Navy and Air Force, 4 April).
The pdf version of the 2016 May RSL table is provided on the compact disk for this RFI Report.

[d] PSV calculated using the RSL Calculator, with current IRIS toxicological values for benzo(a)pyrene and related PAHs (EPA, 2017, *Toxicological Review of Benzo(a)pyrene (CASRN 50-32-8)*, Integrated Risk Information System, Washington, D.C., EPA/635/R-17/003Fc, January, at https://cfpub.epa.gov/ncea/iris/iris_documents/documents/toxreviews/0136tr.pdf.

APPENDIX H

INVESTIGATION-DERIVED WASTE, STANDARD OPERATING PROCEDURE 4.0

Subject: INVESTIGATION-DERIVED WASTE

1.0 PURPOSE AND SUMMARY

This Standard Operating Procedure (SOP) establishes specific management practices for the in-process handling and subsequent disposition of environmental media generated as a result of investigation and removal actions at Redstone Arsenal (RSA), Madison County, Alabama. Investigation-derived waste (IDW) will be handled in accordance with the most recent versions of Alabama Environmental Investigation and Remediation Guidance and Alabama Administrative Code (AAC) 335-14. This SOP serves as an update to IDW plans previously submitted to comply with Alabama Department of Environmental Management (ADEM) Consent Order No. 97-203-CHW for the management of Comprehensive Environmental Response, Compensation, and Liability Act (CERCLA) IDW.

In support of RSA's Installation Restoration Program under the Federal Facilities Compliance Act of 1992 and CERCLA and to meet the requirements of RSA's Resource Conservation and Recovery Act (RCRA) permit, RSA is conducting investigation and removal activities which generate environmental media. The media typically consist of drill cuttings and fluids, monitoring well purge and development water, spent personal protective equipment (PPE), and other inert materials (i.e., plastic, rope, tape, paper, etc.) generated during operations, well installation and sampling activities, remedial actions, and associated site activities. When accumulated, the media must be managed appropriately to minimize the exposure to human health and the environment while adhering to applicable regulatory requirements.

2.0 TABLE OF CONTENTS

3.0 RESPONSIBILITIES

3.1 Quality Control Site Manager

The Quality Control Site Manager (QCSM) is responsible for ensuring that field activities are completed to meet the project objectives, that they are conducted in accordance with the project plans and requirements, and that all activities are performed according to their respective procedures. The QCSM is responsible for ensuring that all site personnel are trained in the procedures, that the procedures are adhered to, and that all activities are documented.

3.2 Field Team

All members of the field team (samplers, technicians, field geologists, engineers, etc.) are responsible for understanding and implementing this field procedure as well as ensuring that all team members also perform work in accordance with this SOP.

3.3 Quality Assurance/Quality Control Manager

The Quality Assurance/Quality Control Manager is responsible for ensuring that this SOP is correctly implemented and that the quantity and quality of field- measurable physical characteristic samples collected meet the requirements of the Site-Specific Field Sampling Plans (SFSP).

4.0 DEFINITIONS

None.

5.0 TEXT

5.1 Required Records and Forms

For a description of required forms, refer to SOP No. 1.0, *Field Documentation*.

- Sample Collection Log (SCL)
- Field Activity Daily Log

- Sample tags/labels and the appropriate forms/documentation for sample shipment
- Material Safety Data Sheets (MSDS)
- SFSP.

5.2 Required Materials, Equipment, or Supplies

- Indelible black ink pens and markers
- Appropriate sample containers
- Insulated cooler and waterproof sealing tape
- Nitrile or latex gloves
- Decontamination equipment and supplies, including rinse bottles and deionized water
- Personal protective equipment (PPE)
- Socket wrench or bung wrench to access drums
- Appropriate equipment and meters for obtaining field measurements as specified in the SFSP (i.e., photoionization detector/flame ionization detector).

5.3 Procedures

5.3.1 Preparation

The following steps must be followed when preparing for management activities of IDW:

- Verify that all personnel have read and understand the approved Site-Specific Health and Safety Plan and have the proper training and certifications required under the Occupational Safety and Health Administration.
- Don the appropriate PPE as dictated by the Site-Specific Health and Safety Plan.
- Document the sampling events, recording the information on the SCL or equivalent form as specified. Document any and all deviations from standard operating procedures on the Field Activity Daily Log and include rationale for changes.

5.3.2 Specific Preparation

The following paragraphs detail the planned methodologies for dealing with environmental media generated during site activities. For the purpose of this document, a site, an area of contamination (AOC), and a solid waste management unit (SWMU) are all synonymous.

5.3.2.1 Initial Handling Requirements

All environmental media will be managed in an effort to minimize exposure to human health and the environment. Typically, the media will be generated as a result of these

major activities: drilling soil borings; installation and development of monitoring wells; and groundwater sampling activities.

In instances where soil borings are advanced, either to retrieve soil samples or to allow for the retrieval of a groundwater sample via a hydropunch or similar sampling device (including obtaining a sample from an open borehole), the following handling protocols for IDW soil will be used:

- All soil cuttings will be placed adjacent to the borehole on plastic or other suitable material capable of preventing contact with the ground surface.
- All cuttings will be covered daily or during rainfall events to prevent contact with moisture.
- Upon completion of the downhole activity (i.e., drilling, groundwater sampling, etc.), the soil cuttings will be placed in open topped 55-gallon drums, labeled, sampled, and properly stored.

In cases where a soil test boring is advanced for the purposes of installing a monitoring well, all environmental media accumulated will be containerized to allow for characterization upon generation and situated at or near the point of generation. As solids are generated, they will first be placed into open-topped 55-gallon drums or other approved containers pending further characterization. Solids may be bulked into larger approved containers situated within the AOC. Liquids may be bulked upon generation unless directed otherwise. All solids and liquids will be separated prior to disposal.

Liquids may be held on site at the AOC or SWMU and are not required to be moved to a separate 90-day storage area. However, either the satellite accumulation restrictions regulating storage of less than 55 gallons or 90-day storage rules would apply to hazardous liquids that remain on the SWMU/AOC. Section 5.3.2.3 further discusses storage requirements. If hazardous liquids are stored on site, the satellite accumulation area or the temporary less-than-90-day storage area must meet ADEM requirements for secondary containment standards as noted in Section 5.3.2.3.

5.3.2.2 Labeling

After each container (i.e., drum, roll-off box, etc.) has been filled, the container and lid, if appropriate, will be labeled with a description of the media (i.e., soil, purge water, decon water, PPE), origin of media (i.e., Soil Boring A- 1, Monitor Well RS-0 1 2, etc.), date the media were placed in the container, site identification (i.e., SWMU or AOC number), date container was sealed and sampled, and a short statement stating that the contents are on hold waiting analytical test results. If the analytical results determine that the container contents are hazardous, a standard hazardous waste label will be placed on each container. The accumulation start date will be the same as the date recorded on the initial drum. A copy of correspondence (email) from ADEM clarifying their position on handling of potentially hazardous wastewater at RSA is provided as an attachment to this SOP. Nonhazardous waste containers may be labeled using a paint pen or other indelible

marker that will not fade when exposed to weather. Hazardous waste containers will be marked with labels and information pursuant 40 Code of Federal Regulations (CFR) 262.34. A record of the number of containers, their contents, and the regulatory status of the waste will be completed at each generation site and will be included in the Field Activity Daily Log before leaving each site.

5.3.2.3 Storage

At the end of each day and/or field activity, all containers will be sealed or covered in such a way to prevent the introduction of rain water or surface runoff. Nonhazardous IDW will either be moved to a central IDW storage area, or, if feasible and in the best interest of operations, nonhazardous IDW will remain in the SWMU/AOC from where it was generated until final disposition is selected.

Within 72 hours of being generated, hazardous solid IDW will be moved to an RSA-approved Satellite Accumulation Area, a temporary 90-day storage area, or a fully permitted waste storage area. Wastewater IDW may be held at the AOC or SWMU in a temporary less-than-90-day storage area or it may be moved to a central 90-day storage area. Any temporary 90-day storage area established on an AOC or SWMU will meet ADEM's secondary containment standards. Wastewater or solid hazardous IDW will be labeled during storage as discussed in Section 5.3.2.2.

Waste may be transported between storage areas when required or in preparation of disposal activities without specific regulatory concurrence (i.e., RSA is not required to obtain specific regulatory approval to transport wastes within the confines of RSA). Drums of hazardous wastewater will be removed from the AOC or SWMU in less than 90 days. All hazardous IDW will be shipped off site or properly treated and managed on site within 90 days of its accumulation start date.

5.4 Characterization of Media

The characterization of the media will be determined by a combination of generator knowledge and use of analytical data obtained during the activity from which the materials were generated. As stated, it is anticipated that specific generation activities will include soil borings, monitoring well installations, and monitoring well purge and development actions. Water obtained from specific monitoring well sampling points (i.e., purge and development water) will be characterized using groundwater sampling data taken from the specific well site from which the water was obtained. Analytical data obtained from a particular borehole reflecting soil contaminant levels will be used to characterize solids generated from that borehole. Other solids (such as rock) will be characterized for disposal based on the analytical results of the soil and water sampled at the specific location where the solids were generated. When appropriate, analytical data will be extrapolated to reflect toxicity characteristic leaching procedure (TCLP) values (i.e., 20x divisor rule for soils). Generator knowledge may be used to evaluate the media potential for toxicity, corrosivity, ignitability, reactivity, and listed waste scenarios.

In the event generator knowledge and data associated with previous site investigations are inadequate to accurately and thoroughly characterize the IDW, waste will be managed as hazardous waste. A representative sample will be retrieved from each waste stream warranting further characterization. In addition, representative samples will be collected from all IDW determined to be nonhazardous based on generator knowledge. These samples will be taken directly from containers after the waste has been generated. The suite of analyses to be run will be determined based on suspected contaminants and any information gleaned from previously available data. Hazardous versus nonhazardous determinations will be made utilizing those parameters outlined in AAC R. 335-14-2-.02, *Criteria for Identifying the Characteristics of Hazardous Wastes and for Listing Hazardous Waste*. More specifically, hazardous characteristics will be determined utilizing the requirements of AAC R. 335-14-2-.02 (1) and 335-14-2-.03. Where listed wastes are expected or where the potential exists, specific analytes (i.e., totals as opposed to TCLP) for the listed compounds will be tested in addition to determining any hazardous characteristics. All sampling and analytical testing protocols will be consistent with ADEM/U.S. Environmental Protection Agency (EPA) requirements and methodologies.

5.5 Management and Disposition

Once adequately characterized, the containers will be labeled as described. U.S. Department of Transportation-approved labels will be used if transportation outside of RSA boundaries is required or anticipated. The media may also be bulked on site (within the staging area) with like waste streams possessing compatible nonreacting characteristics.

5.6 Wastewater

In general, all wastewater generated during the described site activities will most likely be disposed either at an RSA-approved treatment facility or at the wastewater treatment facility currently operated at RSA.

5.6.1 Nonhazardous Wastewater

Upon proper characterization and approval from RSA representatives, wastewater determined to be nonhazardous (Section 40 CFR Part 261) but possessing some level of contaminants can be disposed directly into RSA's sanitary sewer system, where it will ultimately be treated at the RSA wastewater treatment plant (WWTP). The RSA representative will request waste characterization data, approximate volume, and the location of disposal in making the determination to accept sewer discharge. The nonhazardous water will typically be discharged at a manhole(s) located near the generation site.

All discharges will be in accordance with provisions outlined in Division 6, *Water Quality Program*, of the AAC. More specifically, the discharge will not be greater than 5 percent of the average dry weather capacity of the WWTP, greater than 5 percent of the

design capacity of the WWTP, or subject to Section 403.6 of the Federal Water Pollution Control Act. No disposal permit is required as long as the wastewater is discharged in quantities of less than 25,000 gallons per day and the water is nonhazardous (40 CFR 261).

Wastewater generated during site activities and for which analytical tests showed no level of contamination present above approved detection limits will be considered nonregulated. The disposal means and methods of nonregulated waste water are at the discretion of RSA representatives (e.g., storm water system, open ditch, etc.) and do not require regulatory consultation or concurrence.

On a quarterly basis, RSA will submit documentation of all discharges (regulated and nonregulated) to ADEM. The documentation will contain pertinent information regarding the discharge, including, date, time, volumes, analytical data (if available), site, action, etc**.** All discharges to the sanitary sewer system will be coordinated in advance.

5.6.2 Hazardous Wastewater

Hazardous wastewater will be transported, when required, and treated at an off-site wastewater treatment facility when the following conditions are met:

1. The treatment facility meets the definition of a wastewater treatment unit as defined in AAC R. 335-14-1-.02.
2. The treatment facility is capable of (a) rendering characteristically hazardous wastes (AAC R. 335-14-2-.03) nonhazardous or (b) removing listed wastes (AAC R. 335-14-2-.04) from the contaminated media so that the media no longer contain the listed waste for which the media were originally considered hazardous. If after treatment, analytical tests show the listed waste is not present above laboratory detection limits, then the contaminated media will be considered to no longer contain the listed waste and will no longer be considered hazardous.
3. The wastewater treatment facility has been constructed at RSA in conjunction with a removal, interim remedial action, or remedial action at an AOC.

At no time will liquids that possess hazardous characteristics or meet the definition of a listed waste be disposed into the sanitary sewer system, unless the waste is specifically exempt under RCRA, CERCLA, or its applicable or relevant and appropriate requirement without applicable ADEM authorization.

Wastewater determined to be hazardous may be transported between AOCs and within RSA boundaries for treatment/disposition in accordance with the previously outlined provisions without specific regulatory concurrence.

On a quarterly basis, RSA will submit documentation of discharges to ADEM. The documentation will contain pertinent information regarding the discharge including date, time, volumes, analytical (if available), site, action, etc.

All discharges to the sanitary sewer system will be coordinated prior to any discharge.

In the event that RSA does not have a facility on line capable of treating the hazardous wastewater at or around the time of generation, and the water is expected to remain on site for a prolonged period of time (but not to exceed 90 days), the water will be stored in an area with an adequate secondary containment system until an approved treatment system is on line.

Unless specifically mandated by ADEM and EPA, the treatment and disposal of hazardous and nonhazardous wastewater will be performed as previously described. The wastes will be treated and disposed in a timely manner so as to expedite site activities and to ensure the protection of human health and the environment. Except where noted, specific written concurrence from ADEM and EPA prior to those actions previously described is not required.

5.7 Solids

Solids may include soil cuttings, rock, grout, spent PPE, plastic sheeting, rope, unused monitoring well construction materials, and other environmental media generated during field activities. All solids will be containerized at or near the point of generation and staged as described in Section 5.3.2.1. Other specific management practices are described in Sections 5.7.1 and 5.7.2.

5.7.1 Nonhazardous Solids

Soil cuttings and rock determined to be nonhazardous will be staged within the confines of the AOC from which they were generated or stored properly in an RSA-approved storage area. After characterizations (hazardous versus nonhazardous) are finalized and depending upon site conditions, nonhazardous cuttings will be removed from containers and replaced “at or near” the location from which they were derived. “At or near” infers media will be placed as near to their point of origin as is practical. Examples would be placing monitoring well cuttings around the monitoring well from which they originated as opposed to within it. However, when not practical, the media may be centrally located within the confines of the originating AOC in an area of minimal traffic and where the media could be managed in a manner protective of human health and the environment. At no time will contaminated media originating from one AOC be transported to another AOC for placement without prior written concurrence from ADEM and EPA.

In the event that site conditions are not conducive to the replacement of the materials (i.e., restricted space, confined area, etc.), soils and rock determined to be nonhazardous may be disposed into RSA's Solid Waste Disposal Facility-Construction/Demolition Landfill (ADEM Permit No. 45-03) or an approved off-site non-hazardous solid waste disposal facility as long as the following conditions are met:

1. Soils exhibiting contaminant levels below analytical detection limits are considered nonregulated and will be disposed at the discretion of RSA representatives.

2. The soil analytes do not exceed 50 percent of the TCLP analysis for any given compound. A disposal report is submitted within 45 days of disposal that includes a signed copy of ADEM's Solid/Hazardous Waste Determination form and any applicable analytical results.

Other nonhazardous solids such as spent PPE, plastic sheeting, rope, unused monitoring well construction materials, and other environmental media generated during field activities that have been determined to be nonhazardous will be emptied into dumpsters or roll-offs for disposal off site at a permitted solid waste disposal facility.

5.7.2 Hazardous Solids

Hazardous IDW solids can be segregated into two categories for purposes of waste management. The first is strictly IDW soils. Hazardous IDW soils will be immediately handled and stored as hazardous waste while on RSA. The waste soils will be analyzed, profiled, and managed off site at a permitted transportation, storage, and disposal facility for its characteristic and/or listed waste status. The second hazardous IDW solid category is essentially all non-soil-like media, generally anticipated to be in the form of debris and PPE. The soil versus nonsoil differentiation is necessary in order to select the correct treatment and disposal technology. Hazardous nonsoil and debris media can present different analytical and treatment strategies than contaminated soils.

6.0 EXCEPTION PROVISION

None.

7.0 CROSS REFERENCES AND OTHER SOURCES OF INFORMATION

This SOP will be used in conjunction with the following cross references where applicable.

SOP No. 1.0 – Field Documentation

SOP No. 11.0 – Field Generated Records Management

Alabama Department of Environmental Management (ADEM), 2009, Division 14 - Hazardous Waste Program, Revised Effective March.

Alabama Department of Environmental Management (ADEM), 2005, **Alabama Environmental Investigation and Remediation Guidance**, September.

McCoy and Associates, 1995, **RCRA Regulations and Keyword Index**, Elsevier, 1995.

U. S. Environmental Protection Agency (EPA), 1992a, **Guide to Management of Investigative-Derived Wastes**, Office of Solid Waste and Emergency Response, Publication 9345.3-03FS, April 1992.

U. S. Environmental Protection Agency (EPA), 1992b, **Management of Contaminated Media**, Region IV EPA, Guidance Number TSC-92-02, December 28, 1992.

U. S. Environmental Protection Agency (EPA), 1991, **Management of lnvestigative-**

Derived Wastes During Site Inspections, Office of Research and Development, Publication, EPA/540/G-91/009, May 1991.

8.0 ATTACHMENTS

- Attachment 1, ADEM Email Addressing IDW.

ATTACHMENT 1

ADEM EMAIL ADDRESSING IDW

Attachment I
ADEM Email Addressing IDW
RSA IWSAP SOPP 4.0

Kurth, Randy

Subject: FW: Response to ADEM original comments on the IDW discussion {Update}

Importance: High

From: Morrissette, Krishna M [mailto:KMorrissette@adem.state.al.us]
Sent: Wednesday, November 17, 2010 2:54 PM
To: Kurth, Randy
Cc: Davis, Emily; Burton, Don; Hodges, Barry A Mr CIV USA USACE; Shell, Ronald T; Wilson, J Jason; Reese, Dennis
Subject: RE: Response to ADEM original comments on the IDW discussion {Update}
Importance: High

Randy,

Sorry for the confusion on the 90-day storage issue. Here are some comments to further clarify ADEM position on the handling/ staging of potentially hazardous wastewater at RSA:

1. Wastewater can be held at the AOC or SWMU site and does not have to be immediately moved to another < 90 day storage area. The holding area must meet secondary containment standards.
2. It is OK to initially label the wastewater filled drums with the following information.
   - Description of the drum contents (e.g. wastewater from RSA-XXX)
   - Accumulation start date (the date the drum was filled)
   - A short statement that states that the contents are on hold awaiting analytical test results
3. If the analytical results come back noting the drum contents are hazardous, a standard hazardous waste label must be put on the drum noting all required information. The accumulation start date for the standard HW label should be the same date as recorded on the initial drum label.
4. Drums of hazardous wastewater must be removed from the AOC or SWMU in less than 90 days.

Remember that the generator must meet the < 90 day storage rules and regulations (e.g. weekly inspections, training, secondary containment, etc.) while holding the hazardous wastewater drums at the AOC or SWMU site.

As for your response to the example IDW information needed to support generator knowledge determination, it is adequate for our on-site visits. Since it is late in the afternoon for you (EST), I will try to call you to confirm the information presented in this email. Thanks again for your help in this matter, Randy!

Sincerely Yours,

Krishna "Kel" Morrissette
ADEM - Land Division: Facilities Engineering Section
Work: (334) 394-4335
Fax: (334) 279-3050
email: kmorrissette@adem.state.al.us

APPENDIX I

CONSTRUCTION QUALITY ASSURANCE PLAN

Construction Quality Assurance Plan
RSA-271, Former Boiler House, Building 7729,
Operable Unit 10
U.S. Army Garrison-Redstone
Madison County, Alabama
EPA ID No. AL7 210 020 742

Prepared for:

Mission & Installation Contracting Command
ATTN: MICC Center – FSH
2205 Infantry Post Road
Fort Sam Houston, Texas 78234-1361

Prepared by:
Aptim Federal Services, LLC
11400 Parkside Drive, Suite 400
Knoxville, TN 37934

Contract No. W91ZLK-13-D-0018
APTIM Project No. 501021
Task Order 0003

November 2021

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I1.0 Introduction

This construction quality assurance plan (CQAP) presents the overall program for construction quality assurance (CQA) to be implemented by the contractor during corrective measures implementation (CMI) activities at RSA-271. This document establishes a program to comply with requirements established in the CMI work plan and those of the Alabama Department of Environmental Management (ADEM). The scope of work for the project is detailed in the CMI work plan for RSA-271, to which this CQAP is an appendix.

I1.1 Overall Directive

The procedures and practices set forth in the CQAP should be adhered to and specifically applied to all quality-related work on the project. It is the responsibility of all personnel performing work on the project to be familiar with and implement the technical requirements referenced in this CQAP or otherwise specified for the project, as included in the CMI work plan.

Conformance to the requirements of this CQAP will provide results that will verify that the contract, when completed, will conform to the specified requirements and be documented by defensible evidence that the work performed meets or exceeds the standards set forth for the project.

I1.2 Project Background

RSA-271 is located in the southeastern quadrant of Redstone Arsenal (RSA) between Huntsville Spring Branch and the Tennessee River. RSA-271 occupies approximately 0.25 acre and is situated in the southeastern part of the Missile Production Complex (CMI work plan Figures 1-1 and 1-2). A site map is included as CMI work plan Figure 1-2. The RSA-271 site is located within the former Redstone Arsenal Rocket Engine North Plant. Groundwater beneath the site (including the saturated zone soils) is included in the RSA-146 groundwater unit.

Prior to Missile Production Complex construction in the 2010's, the site was mostly open scrub/grassland with scattered areas of small trees and planted pines along its eastern and northwestern boundaries. There are no remaining legacy buildings or structures within the site (CMI work plan Figure 1-2). As is typical of developed areas within RSA, the former buildings at RSA-271 are topographically higher than the surrounding forest land and swampy areas. No permanent surface water or other aquatic habitat features exist within the boundary of RSA-271. A large wetland/pond is located across Eagle Road approximately 500 feet south and east of the site. The entire site lies above the 100-year floodplain.

According to the RSA Master Plan, the land use at RSA-271 is designated as industrial. RSA-271 is located in the Spring Branch land-use district in the southeastern portion of RSA. No surface water is present on the site. The current use of RSA-271 is not residential, nor is it expected to be in the future.

RSA records indicate that Building 7729 was constructed as Army Building B-529 in 1942. Initially, the building housed a steam boiler fueled from a 2,800-gallon underground storage tank (UST) formerly located east of the building. A small oil/water separator (OWS) consisting of a blow-off/hot well pit was located at the west end of Building 7729. RSA records indicate that this system was removed from use but left in place, and a larger, high-capacity blow-off/hot well was installed south of Building 7729 in 1974. The junction of both OWS systems with the sanitary sewer system was located northwest of the building. The boiler was utilized until 1982. From 1982 to demolition in the mid-1990s, the building was used for storage (Shaw Environmental, Inc. [Shaw], 2005).

Building 7729 was still in place but emptied of all equipment during a 1996 environmental baseline survey (EBS) (Conestoga-Rovers and Associates, 1996). The Phase I EBS summary for Building 7729 assumed that a fuel storage tank existed, but the report notes that there was no visible evidence of the UST when the inspection was performed. RSA records list the 2,800-gallon UST as being removed in September 1998. The building is not present in the 2000 aerial photograph, indicating demolition sometime between 1996 and 2000. During a 2004 visual survey associated with a limited site assessment, the building, original blow-off/hot well, and sewer junction were no longer present. However, the replacement blow-off/hot well was still in place. During ongoing Missile Production Complex construction activities in October 2011, the replacement blow-off/hot well overflow pit was inadvertently breached, and the contents were released onto the ground surface. A spill response for a petroleum release was completed, including collection and analysis of post-response samples. The data showed that the spill response had adequately removed the assumed petroleum contamination and prevented further release to the site environment. Results from the post-response samples were below preliminary screening values for the petroleum compounds.

RSA-271 was established as a solid waste management unit following the RSA-146 potential source area (PSA) investigation (Shaw, 2005). As a result of the PSA investigation process, the Army recommended the creation of RSA-271 to facilitate a more focused and thorough evaluation of the data to determine whether chemicals used at RSA-271, including fuel and compressor oils, contributed to environmental contamination. At the time, based upon its limited use as a boiler house, RSA-271 was assigned as an RSA petroleum, oil, and lubricants program site. However, in accordance with the Permit (ADEM, 2020), this site was evaluated more

completely during completion of the Resource Conservation and Recovery Act facility investigation (RFI) (CB&I Federal Services LLC [CB&I], 2017).

The RFI report for RSA-271 was completed in 2016 (CB&I, 2017), and the report was approved by ADEM on April 16, 2020. The report recommended corrective measures to address chemicals of concern (COC) in soil (1-methylnaphthalene, methylene chloride, and naphthalene) and COCs in groundwater (manganese, trichloroethene [TCE], 1-methylnaphthalene, 2-nitrotoluene, and perchlorate) at RSA-271. A weight-of-evidence approach was used to evaluate the single detection of methylene chloride in soil present at a concentration greater than its dilution-attenuation factor of 4 soil screening level. Based on this evaluation, methylene chloride is determined to not pose a leaching threat.

For 1-methylnaphthlene and naphthalene, an Alabama Risk-Based Corrective Action Risk Management-2 fate and transport evaluation was performed for the RSA-271 site. This evaluation included modeling to determine if an action is needed for the small volume of soil around the former UST pit with concentrations of 1-methylnaphthlene and naphthalene that exceeded the RSA generic dilution-attenuation factor 4 soil screening levels. The modeling concluded that concentrations of 1-methylnaphthlene and naphthalene in soil do not pose a threat to groundwater quality. However, subsurface soil samples will be collected during the initial phases of CMs to determine, by synthetic precipitation leaching procedure, the need for a soil excavation. The documentation to complete the excavation is included in this CMIP.

The development of corrective measures was warranted for five contaminants in groundwater (manganese, 1-methylnaphthalene, 2-nitrotoluene, perchlorate, and TCE). No further action will be required for soil at RSA-271. Additionally, a number of COCs in groundwater did not pose unacceptable risk to potential receptors but exceed screening values and will be included in groundwater monitoring in the corrective measures to meet ADEM's concern that the concentrations are not increasing to where they could pose unacceptable risks.

The Army has elected to perform corrective measures to address contamination in the soil and underlying groundwater at RSA-271. The selected corrective measures consist of excavation of contaminated soil and off-site disposal, monitored natural attenuation (MNA) and land-use controls.

I1.3 Objectives of the CQA Program

The objectives of the CQA program are to provide a system of procedures, practices, guidelines, and controls which, when implemented, will provide the confidence that project activities are accomplished in accordance with the specified contracts, design criteria, plans, drawings, and the

CMI work plan developed during implementation of the corrective measures. This CQAP establishes requirements for developing the overall site-specific construction quality control (CQC) system to be implemented at RSA-271. The CQAP will be implemented during all phases of the project, including preliminary site activities, remediation, and close-out activities.

The Army will observe the work during the contractor's performance and approve the work upon acceptable completion.

The CQAP is also applicable to off-site suppliers of equipment or services to the project that could affect the quality of the CMI. In particular, the following items must be adhered to during the CQA activities:

- Guidelines and requirements prepared and documented in the CMI work plan
- Construction verification as work is performed, by inspection and verification testing, so that the design features are implemented as intended
- Evaluation of variance to the design that may occur during construction and remediation and its effect upon system performance
- Complete documentation prepared and maintained during and after construction and remediation so that it can be demonstrated that the design has been implemented and the performance requirements have been met.

I1.4 Presentation of the CQAP

This CQAP is designed so that the CQC activities for all portions of the remediation are executed and managed from a common set of quality objectives and practices as described in the CQC plan and the installation-wide (IW) quality assurance program plan (QAPP) (Shaw, 2013 and as updated). The CQA and CQC activities, as described herein, serve as the minimum requirements to verify that all work is in compliance with the quality requirements set forth in the CMI work plan and consistent with the local, state, federal, and other appropriate regulatory agencies for the types of environmental activities performed.

I2.0 Responsibility and Authority

Aptim Federal Services, LLC (APTIM) will perform the CMI for RSA-271. RSA will observe the work as it is performed to ensure compliance with the CMI work plan. Observation by ADEM personnel will be dependent on the phase of the work being performed.

It is the responsibility of all project personnel to report activities that could adversely affect the CQC requirements set forth by the contract documents. The dedicated Contractor Quality Control Systems Manager (CQCSM) is specifically responsible for identifying, reporting, and documenting activities affecting quality and for verifying correction of materials and activities that do not conform to the specified contract requirements. The Site CQCSM will maintain a close working relationship with the APTIM Project Manager (PM) and RSA, keeping them advised of all situations which, if not corrected or controlled, could affect the resulting quality of the project.

APTIM will designate an authorized representative to be responsible for CQA, referred to as the Site CQCSM. RSA will ultimately be responsible for providing the relevant documentation to the oversight agency (ADEM). The contractor will be responsible for furnishing appropriate documentation (outlined in this CQAP) to RSA for submittal to the oversight agencies, as required.

I2.1 CQA Organization and Key Elements

The APTIM PM will be responsible to ensure the execution of the CQA duties for RSA-271, which will be performed by the Site CQCSM. APTIM, including its subcontractors, will be responsible for field activities and laboratory testing requirements for the project CQC. The responsibility of key personnel involved in the CQA and CQC activities are described in Sections 2.1.1 through 2.1.6.

I2.1.1 APTIM Project Manager

The PM has the overall responsibility to ensure the execution of the work to be performed by the contractor, including efforts to ensure compliance with the requirements of ADEM. Among other duties, the PM will coordinate all financial and project-required resources (technical as well as administrative) necessary for the implementation of the project. The PM will maintain overall responsibility of the project through coordination activities with the contractor, ADEM, and the Army.

The PM shall verify that the corrective measures have been implemented in accordance with the CMI work plan. The PM has the authority to select and dismiss organizations charged with

implementation of the corrective measures and is vested with the authority to stop work if conditions adverse to quality are persistent and need to be corrected before proceeding further.

I2.1.2 Regulatory Oversight Agency (ADEM)

The primary regulatory oversight agency for the RSA-271 CMI is ADEM. The oversight agency will provide review and comment on the CQAP to ensure that the proposed CQA program will provide for sufficient confirmation that work is being performed as intended. The regulatory oversight agency has the responsibility to review CQA documentation during and upon completion of the corrective measures to confirm that the CQAP has been followed and construction/remediation has been performed in accordance with the regulatory requirements.

I2.1.3 APTIM Site CQCSM

The APTIM Site CQCSM will be responsible for the review and approval of the equipment and materials supplied by the contractor (including its subcontractors). The Site CQCSM reports directly to the contractor director of quality assurance (QA)/quality control (QC). The work that the Site CQCSM produces is subject to the review and approval of the PM, the Program CQCSM, and the Project Engineer.

A few deviations from the CMI work plan are not uncommon during the implementation of corrective measures. As such, activities may need to be adjusted accordingly during the progress of construction and remediation. The Site CQCSM may be requested to change some aspects of the design and/or CMI work plan if unexpected conditions (e.g., a change in site conditions, unanticipated logistical problems, change in construction/remediation methodology, or lack of availability of certain materials) are encountered during the construction work. Accordingly, the Site CQCSM will be responsible for preparing the appropriate revisions and providing necessary feedback to the PM or Director of QA/QC.

The Site CQCSM is responsible for coordinating all required field activities and laboratory CQC testing activities, including sample collection and shipment and verification of the test results. The results will be documented on the daily construction log. Additional responsibilities include preparing addenda to CQAP and formulating corrective actions when variance or nonconformance is detected or required.

I2.1.4 CQC Analytical Laboratory

In accordance with the contract documents, CQC activities for groundwater will be performed by a subcontracted laboratory. The testing laboratory must have their own internal QC procedures to ensure that laboratory analyses conform to the appropriate regulatory requirements and applicable testing standards. The CQC laboratory is responsible for ensuring that analyses are

performed in accordance with applicable test methods and standards for following internal QC procedures, maintaining sample chain-of-custody records, and reporting data. In addition, the CQC laboratory must be willing to allow announced or unannounced inspections by authorized project personnel, including representatives from RSA and ADEM, in order to observe the sample preparation and analysis procedures. The laboratory must be willing to accommodate such inspection as long as the observer does not interfere with the testing process.

I2.1.5 APTIM and Subcontractors

APTIM has the overall responsibility for conducting the remediation in accordance with the approved CMI work plan. APTIM, including its subcontractors, must perform CQC tests, as required by the CMI work plan, during project remediation activities; provide CQC documentation as specified; and report variances and nonconformances as outlined in this CQAP.

I2.1.6 CQC Personnel

Field QA/QC personnel are individuals designated by APTIM and its subcontractors and whose duty it is to ensure that products and services are provided to the RSA in accordance with the CMI work plan.

I2.2 Qualifications

CQA and CQC activities will be accomplished by appropriately qualified personnel. Everyone shall understand and enforce the specified quality requirements and recommend improvements in processes and/or services that could affect the cost, schedule, and quality of the project in a positive manner.

The key personnel involved in the CQA/CQC program and their minimum recommended qualifications are provided in the following table:

Key Personnel in CQA/CQC Organization	Role/Minimum Qualifications Requirements
APTIM PM *Mark Shoemaker*	The specific individual(s) to certify that the construction activities have been completed in accordance with project design CMI work plan.
Project Engineer *Ken Hurley*	The individual who has knowledge of the design and contract requirements.
QA/QC Manager *TBD*	Independent supervisor of Site QA/QC technicians.
Site Manager *TBD*	The individual implementing the CMI work plan. U.S. Army Corp of Engineers CQA trained.

Key Personnel in CQA/CQC Organization	Role/Minimum Qualifications Requirements
Site CQCSM *Brian Rhodes*	Responsible for the review and approval of the equipment and materials and coordinating all required field and laboratory CQC testing activities, including record keeping, sample collection, and shipment. Five years project work; prefer college degree in science or technical field.
APTIM Field QA/QC Personnel *TBD*	Designated contractor and/or subcontractor personnel (or independent third parties) to perform specific CQC testing. No training required other than in-the-field training for assigned tasks.

TBD – To be determined.

I2.3 APTIM Personnel Training

Personnel assigned to the project, including subcontractors, are trained to ensure competence commensurate with the responsibility and qualifications necessary to perform the assigned tasks. In addition to education and experience, job-specific training may be required to qualify individuals to perform certain activities. The APTIM PM and Site CQCSM will review and document the personnel qualifications and training to verify compliance with the subcontract requirements.

All personnel will be trained in accordance with Occupational Safety and Health Administration Section 1910.120. Project personnel will receive an orientation to the CMI work plan as appropriate to their responsibilities before participation in project activities.

The APTIM PM and Site CQCSM will review the qualifications and training of all personnel assigned to the project. Training and qualification records will be maintained at the project site available for review. Training of site personnel will be verified and documented as applicable to the work to be performed. The Site CQCSM will monitor the training activities to verify all required training is completed and documented and current records are maintained.

Training will include all phases of the work as necessary and will be commensurate with the complexity of the activities being performed. Training methods may include formal classroom training, required reading, on-the-job training, or combinations of these methods. Training procedures will be reviewed and approved by qualified contractor project personnel.

Training programs are conducted according to organizational needs and policies so that personnel have the following qualifications:

- Acceptable understanding of the safety consideration of the work tasks
- Knowledge of the processes adequate to perform assigned tasks

- Working knowledge of the project or facility basis requirements
- Understanding of systems, terminology, reasons for performance of specific control functions, and acceptance and rejection criteria for the work
- Consequences of inadequate quality attainment.

The training program will be evaluated to determine the effectiveness of the program and instruction. If it is determined that the program content, instructor capabilities, or other conditions require changes, the program will be updated at that time. At a minimum, the training will be reviewed as part of the management assessment.

Training Records. APTIM will maintain qualification and training records for each employee. Training records should include all documents that establish the employee's capabilities, including outside training and training performed by approved training organizations. The Site CQCSM will verify compliance with the project requirements.

I2.4 Communication Within the CQA Organization

Communication between the CQA program participants includes the exchange of information which allows work to proceed and the required reporting so that activities can be reviewed. Construction documents, inspection reports, audit reports, verification test results, and daily construction logs must be timely so that reviews and evaluations can be performed by all the parties responsible for execution of the work.

CQA personnel, the PM, and the contractor and its subcontractors must communicate as required and as addressed in this CQAP to maximize the efficiency and effectiveness of the CMI and minimize variance or nonconformance.

I2.5 CQA Meetings

CQA meetings will be held throughout the progression of construction/remediation activities on an as-needed basis. Progress meetings will be documented in the form of meeting minutes prepared by the Site CQCSM and maintained in the on-site CQA files.

I3.0 Contract Scope of Work

The complete and detailed scope of work for the planned construction/corrective measures activities is presented in the CMI work plan and the supporting documents. Additionally, the CMI work plan provides the proposed schedule and sequencing of the activities. This chapter provides a general overview of the activities and an outline of the CQC testing requirements referenced in the CMI work plan. Chapters 4.0 through 10.0 present the necessary supporting aspects of the CQC/CQA program that must be implemented to ensure that the overall objectives of the program are met and provide evidence of compliance with all applicable project and regulatory requirements.

I3.1 Proposed Work Activities

The general scope of work for the corrective measure activities at RSA-271 includes the following:

- Procurement and subcontracting
- Preliminary site activities:
  - Mobilization
  - Access to RSA
  - Site controls
  - Utility clearance and marking
- Monitoring well installation and development
- Excavation of contaminated soil
- Post-excavation soil confirmation sampling and analysis
- Waste characterization sampling
- Transport and disposal of excavated soils contaminated with PAHs as nonhazardous waste (Subtitle D landfill)
- Site restoration, including application of backfill and topsoil and revegetation with approved grass mixtures
- Surveying
- Groundwater sampling
- CMI construction report

- Annual groundwater performance monitoring and reporting to evaluate the effectiveness of MNA in the groundwater plus supplemental sampling (quarterly or semiannual) as required to provide additional data needed by the Army and the CMI contractor to evaluate the corrective measures.

- Demobilization

- Groundwater performance monitoring and reporting to evaluate the effectiveness of MNA in the groundwater until COCs attain cleanup goals (CG) for three consecutive years.

I3.2 CQC Requirements and Responsibilities

Sections 3.2.1 through 3.2.3 present a summary of the CQC testing requirements and responsibilities of the Site CQCSM during implementation of corrective measures at RSA-271. The information presented herein is intended only to provide an overview of the requirements; the complete and full details of the planned work are contained in the CMI work plan and supporting documents.

I3.2.1 Preliminary Activities

Preliminary activities include mobilization, requirements for base access, site control, digging permits and utility marking, vegetation clearing, construction and development of new overburden monitoring wells, obtaining supplies for groundwater sampling, and baseline groundwater sampling. The digging permit will be obtained from the Directorate of Public Works prior to commencement of the CMI. During these activities, the Site CQCSM will be responsible for reviewing purchase orders and packing slips to ensure all materials received are in accordance with the CMI specifications. Site controls will be enforced in accordance with the site-specific safety and health plan. A licensed land surveyor will be subcontracted to survey the location of the new overburden monitoring well. A licensed driller will be subcontracted to install and develop the new overburden monitoring well in accordance with project specifications.

I3.3 Excavation of Contaminated Soil

The proposed excavation area and the site layout are shown on Figure 4-2. The excavation is 10 feet long by 20 feet wide (200 square feet). The approximate volume of the contaminated soil is 4,000 cubic feet or approximately 148 bank cubic yards of soil. Assuming a bulking factor of 1.3 from excavating, the disturbed or loose volume will be approximately 200 loose cubic yards.

When the excavation areas have been cleared and the excavation limits have been demarcated, APTIM will begin excavating from the designated areas using a backhoe or excavator. The excavations will extend to a depth of 20 feet. All excavations will be conducted in accordance

with the Safety and Health Regulations for Excavations (Occupational Safety and Health Administration 29 Code of Federal Regulations Part 1926 Subpart P). Excavations over 4 feet deep will be shored, sloped (1½:1, horizontal:vertical), or benched as required. Spoils will be placed a minimum of 3 feet from the edge of the excavation. Loose soil or rocks will be removed from the sides of excavation walls. Personnel will not enter any excavation over 4 feet deep that is not properly sloped or benched.

Following excavation, soil confirmation samples will be collected from the sidewalls of each excavation to verify that the CG for PAHs have been achieved. The excavated material will be temporarily stockpiled or staged prior to waste characterization sampling and off-site disposal. The excavated material will be segregated (contaminated versus uncontaminated), staged on impervious material such as plastic sheeting, and covered with waterproof material (i.e., tarpaulin or 10-mil plastic sheeting). Containment will control runoff, leaching, or fugitive dust emissions. Measures will be taken to prevent any surface runoff from entering into or washing away from the stockpile. The excavated areas will be adequately secured from the public and filled as soon as possible.

I3.3.1 Groundwater Monitoring

Groundwater monitoring will commence with baseline groundwater sampling. The rationale for well selection, frequency of sampling, procedures and analytical parameters, and reporting is discussed in detail in the Groundwater Monitoring Plan for this site (Appendix F). The samples will be collected using low-flow purging and sampling techniques. The groundwater monitoring samples will be analyzed for the same analytes and field parameters as the baseline sampling event. The samples will be collected using low-flow purging and sampling techniques. The samples will be collected per Standard Operating Project Procedure (SOPP) No. 7.0, *Groundwater Sampling* (Shaw, 2013).

Groundwater monitoring will be conducted annually and continue until the groundwater analytical results indicate the CGs for the groundwater COCs at RSA-271 have been attained for three consecutive years. At that time, the Army will request a permit modification to cease groundwater monitoring.

In addition to the annual sampling events, supplementary sampling events will be conducted as needed to provide additional data needed by the Army and the CMI contractor to evaluate the corrective measures. These supplementary sampling events could occur as frequently as quarterly. Six (three existing and three new) monitoring wells are planned for sampling during the baseline event as shown on CMI work plan Figure 4-1. The same six monitoring wells are proposed for annual sampling following the baseline sampling event (CMI work plan Table 4-1).

All groundwater samples collected as part of the CMI activities for RSA-271 will be analyzed for the same analytes and field parameters as the baseline sampling event. These data will be included as part of the scheduled annual reports.

The data collected from the baseline groundwater sampling event and subsequent groundwater sampling in the overburden will be used to evaluate the effectiveness of the corrective measures. After the concentrations of COCs for action (and daughter products) in all monitoring wells decrease below the CGs for three consecutive years, the Army will request a permit modification to cease groundwater sampling.

I3.3.2 Final Inspection and Demobilization

When planned corrective measure activities have been completed, temporary field structures will be removed and disposed off site. A final inspection of the project site will then be conducted in accordance with the requirements outlined in this CQAP.

After the completed work has been accepted by RSA, all personnel and equipment will be demobilized from the project site. During these activities, the Site CQCSM will be responsible for the following tasks:

- Oversee the removal of temporary field structures.
- Participate in final field inspection and note deficiencies that require corrective action.
- Coordinate implementation of corrective actions and arrange for reinspection.
- Submit final approved inspection report to RSA.
- Oversee demobilization activities.

I3.4 Additional Considerations

The information contained in this chapter only represents an overview of the proposed fieldwork activities and is intended to serve as a guide to the complete details of work as included in the contract CMI work plan and other supporting documents (i.e., data collection QA plan, etc.). As noted previously, the remaining chapters of this CQAP contain discussions of the additional components of the overall CQA/CQC program, which are implemented to ensure the generation of defensible evidence of compliance with contract and regulatory requirements.

I4.0 Document Control

The CQAP is a controlled document, and measures are included to maintain the currency and the use of the plan so that the CQC functions defined within are in accordance with the latest specified requirements. Distribution of the plan is controlled so that all revisions to the plan are issued to the plan holders and the superseded requirements revised accordingly in the existing plans.

Issuance and distribution of the plan will be controlled by the contractor PM or his/her designee, the document controller. The plan will be transmitted to each plan holder on the distribution list. The transmittal document will reference the assigned document control number, which will appear in the top right corner of the transmittal letter included in each document. The assigned number will be kept on a log maintained by APTIM's PM's designee in the home office. Copies will be maintained at specific locations and available to the individuals performing the work.

Revisions to the plan will be made by sections or by the addition of supplements or amendments and noted with change pages or a new final or revised document. All accepted revisions to the plan will be transmitted to the plan holders according to the distribution list. Each individual or organization designated as a plan holder will be responsible for updating their copy of the plan.

I4.1 Documentation

The APTIM PM will provide a document control system to provide measures for the control of issuance, distribution, storage, and maintenance of documents relating to quality, including those from the contractor and its subcontractors and other vendors or suppliers.

Preparation, review, issuance, and revisions to documents affecting construction quality will be controlled so that the specified contract, regulatory, and permitting requirements are clearly defined and made available to the personnel performing the work. Such documents may include but not be limited to the following:

- Correspondence
- Drawings
- Procedures
- Plans
- Reports
- CMI work plan.

The contractor PM or his/her designee will review the documents to verify inclusion of the appropriate QA requirements.

I4.2 Daily Construction Log

Preparation and Submittal of Daily Construction Log. CQC reporting will be addressed in the daily construction log, and the contractor will document all project activities as required by the contract. The log will cover both conforming and nonconforming work and include but not be limited to the following:

- Weather conditions
- Site instructions
- Nonconforming conditions
- Results of inspections and tests
- Types of defects/causes for rejection
- Corrective actions proposed/taken
- On-site personnel/major equipment log
- Delays and causes
- Verbal instructions.

A typical daily construction log is included as Table I.4-1. RSA will be provided a weekly submittal of daily construction logs throughout the duration of the project.

I4.3 Records

I4.3.1 Evidence of Contract Compliance

Records will be prepared to furnish documented evidence that design, construction, and operation activities, including laboratory analysis, are in compliance with the quality requirements of the contract. The records will be consistent with the applicable sections of the project technical CMI work plan and may include one or more of the following:

- Daily construction logs
- Technical reviews
- Inspection and test reports
- Audit reports
- Monitoring and surveillance activities
- Personnel qualifications
- As-built drawings
- Nonconformance reports (NCR)/corrective actions
- Design documents
- Laboratory analyses reports
- Other specified documents.

I4.3.2 Storage of Field Records

Copies of field records will be maintained and stored at a location designated by the PM until turnover as specified by RSA. On-site records will be readily retrievable for review and audit purposes by ADEM and RSA. The records will be controlled so that the possibility of loss, damage, or other detrimental conditions of the records is avoided. The original project documents will be stored at the contractor's home office.

I4.4 Project Submittals

Project submittals include documents generated or revised in the contractor's home office or in the field site office at RSA. Project submittals will have tracking numbers issued with each new or revised document. In addition, project submittals specified in the contract documents and CMI work plan will be prepared by APTIM and submitted to the Site CQCSM. The APTIM PM is responsible for the preparation and maintenance of the specified submittals for the project.

A master set of as-built drawings will be kept at the contractor's field office and marked periodically with variances from the contract documents. Two sets of as-built drawings will be maintained at the site. Each deviation will be identified with the appropriate modifying documentation. These as-built drawings include buried or concealed structures and utility features revealed during the course of the site work. A copy of the marked set of prints and updated design drawings, if required, will be submitted.

I4.4.1 Document Submittal Register

The project submittal register will be maintained in the APTIM home office or field site office at RSA. Submittals returned unapproved or with comments requiring revisions will be so noted on the submittal register and re-entered as revisions. The APTIM Project Engineer or his/her designee will monitor the submittal register to verify submittals are being controlled, scheduled and tracked, and the status kept in an effective manner. The project submittal register will be updated continuously, as applicable, and reviewed by the Project Engineer or his/her designee to determine the status of the submittals and compliance with the project schedule requirements.

I4.4.2 Submittal Preparation and Transmittal

Submittals will be prepared by the APTIM PM or his/her designee. Submittals from subcontractors or vendors will be reviewed by the APTIM Site CQCSM prior to transmitting the submittals to the PM and Project Engineer or designees. All appropriate information will be completed prior to transmittal of the submittals. Submittals will be scheduled to coincide with the needed dates and adequate time allowed for review and approval in accordance with the contract requirements. The submittals will be reviewed for conformance to specified

requirements, completeness, and accuracy. Submittals requiring modifications or changes will be returned to the APTIM PM or his/her designee for corrective actions and resubmitted for review.

I4.4.3 Resubmittals

Submittals which are not approved by the Site CQCSM or returned with comments which require resubmittal for approval will be processed in the same manner as the original submittals. The submittal number used for the original submittal will be used for each resubmittal followed by a numerical notation indicating the revision. The resubmittals will be re-entered on the project submittal register with the new revision number.

I5.0 Nonconformances and Corrective Actions

This chapter addresses the procedure for reporting nonconformances and corrective action for variance from the contract documents.

I5.1 Nonconformance Report

Work, field testing, laboratory testing, or materials not conforming to the CMI work plan or contract requirements, including noncompliances and deficiencies identified by RSA, will be documented on an NCR (Table I.5-1). At a minimum, the NCR will detail the nonconforming conditions, recommended corrective action(s), and disposition of the corrective action(s). The contractor will immediately correct noncompliances or deficiencies identified by the Site CQCSM. A master log of all NCRs will be kept by the Site CQCSM for review by the PM. All NCRs will remain open until the nonconforming condition has been satisfactorily resolved and verified as acceptable by the QA/QC Director.

I5.2 Identification of Nonconforming Items

Items identified as nonconforming will be documented on the NCR, which, as applicable, will include the following:

- Description of nonconforming item or activity
- Detailed description of nonconformance
- Cause of nonconformance
- Referenced criteria
- Recommended disposition
- Disposition and verification of corrective action
- Responsible organization.

I5.3 Nonconformance Tracking Register

Each identified nonconformance will be documented on the sample NCR tracking register (Table I.5-2) which, at a minimum, will include the following information:

- NCR tracking number
- Issue date
- Distribution parties
- Individual/organization assigned responsibility
- NCR closed-out date and initial of party responsible for closure.

The Site CQCSM is responsible for maintaining the NCR tracking register and ensuring that the corrective actions were implemented and verified prior to closing the NCR. RSA will be notified

in advance of verification of the corrective actions to permit their participation in the inspections and acceptance of the results prior to closing the NCR.

I5.4 Control and Segregation

Nonconforming materials or items will be controlled to prevent inadvertent use or further processing which would cause the nonconforming condition to be inaccessible for correction. All items identified as nonconforming will be clearly identified and segregated from acceptable items except where size, installation status, and other conditions would make it impractical to segregate from conforming items. When nonconforming items are not segregated, they will be identified and clearly marked so that they may be easily recognized as nonconforming to prevent further activities prior to the implementation of the corrective action(s).

I5.5 Disposition

The disposition of NCRs will include the necessary actions required to bring the nonconforming condition to an acceptable condition and may include reworking, replacement, retesting, or reinspection. Implementation of the disposition will be in accordance with the original procedural requirements, a specific procedure, or other acceptable written instructions by the QA/QC Director.

I5.6 Documentation

Notifications of noncompliance and the proposed corrective actions will be documented on an NCR and processed in accordance with the provision described in this section. Corrective actions will be implemented upon receipt of the notification. The NCR will remain open until the noncompliance is resolved.

I5.7 Corrective Actions

In addition to resolving identified nonconforming conditions, corrective actions will also address the cause of adverse conditions contributing to the nonconformance and establish methods and controls to preclude the recurrence of the same or similar types of nonconformances.

The Site CQCSM will track corrective actions to identify trends in the causes of the nonconforming conditions and initiate necessary actions to prevent recurrence. Additionally, the Site CQCSM will monitor the corrective actions to verify that corrective actions were properly implemented and accepted and the NCR closed.

I5.8 Stop Work Notice

Nonconforming conditions that affect the quality of the project, threaten safety, or cause an environmental threat will be stopped through the use of a stop work notice (Table I.5-3). Stop work notices may also be issued in the event of insufficient corrective actions resulting in

recurring nonconforming work. The issuance and tracking of stop work notices will be documented on a stop work notice log (Table I.5-4) to be maintained by the APTIM PM (or his/her designee).

I5.9 Conflict Resolution

Conflicts arising from nonconformance and corrective actions that cannot be resolved at the project management and QC levels will be directed to successive levels of management as necessary to obtain resolution. The levels of management will include the Site CQCSM, QA/QC Director, and PM. All conflicts will be resolved within the specified requirements of the contract and the governing regulatory documents.

I6.0 Procurement Control

This chapter addresses the procedure for ensuring that procured items and services meet established requirements and perform as specified within procurement standard operating procedures.

I6.1 Introduction

Prospective suppliers will be evaluated and selected on the basis of the specified criteria. The contractor will ensure that approved suppliers can provide acceptable items and services as required by the contract. The Project Engineer will review and approve all materials and supplies that may affect quality of the project. Upon approval of purchase requisitions, the Site CQCSM will receive a copy of the approved purchase requisition. When materials and supplies arrive at the project site, the Site CQCSM will be responsible to ensure the items and services meet the requirements listed in the purchase requisition and that no items are installed prior to approval of applicable submittals.

The procurement details include provisions for the following, as applicable to the scope of work or services:

- ***Scope of Work.*** A statement of the scope of work to be performed by the subcontractor will be in the procurement documents.
- ***Technical Requirements.*** Technical requirements will be specified. Where necessary, these requirements will be specified by reference to CMI work plan, codes, regulations, procedures, QA program documents, and statement of work requirements that describe the services to be furnished. The procurement documents will provide for identification of inspection, verification, and acceptance requirements for monitoring and evaluating the supplier's performance.
- ***Quality Assurance Program Requirements.*** Procurement documents will require that subcontractors have a documented quality system that implements portions or all of the requirements of this plan, as applicable. The extent of the suppliers' quality system will depend on the type and use of the service being procured.
- ***Right of Access.*** At each tier of procurement, the procurement documents will provide for access to supplier's facilities and records for inspection or audit by the contractor or its authorized representative.
- ***Documentation Requirements.*** Procurement documents at each tier of procurement will identify the documentation required to be submitted to the contractor for information, review, or approval and the time of submittal. The

retention times and the disposition requirements for specific quality records will be prescribed.

- ***Questionable or Unusable Data.*** The procurement documents will include requirements for reporting and approving disposition of questionable or unusable data.

I6.1.1 Review of Procurement Documents

The Site Supervisor will ensure that site-initiated procurement documents and changes transmitted to the prospective supplier include adequate requirements, performance standards, and quality criteria. The purchase requisition will then go through the proper approval process including the APTIM PM, the Project Engineer, and the Procurement Leader.

The review of changes and their effects will be completed prior to transmittal to the prospective supplier. This review will include the considerations that the appropriate requirements are specified and additional or modified performance criteria are determined, and analysis of exceptions or changes requested or specified by the supplier.

I6.1.2 Source Evaluation and Selection

The selection of suppliers and subcontractors will be based on an evaluation of their capability to provide items and/or services in accordance with the specified requirements. Measures for evaluating and selecting procurement sources will be documented and may include one or more of the following:

- Evaluation of the supplier's history of providing an identical or similar service, which reflects the current capability
- Supplier's current QA records supported by documented qualitative and quantitative information that can be objectively evaluated
- Supplier's technical and quality capability as determined by a direct evaluation of their facilities and personnel, an evaluation of the effectiveness of their implementation of their quality system, and, in the case of subcontracted analytical data acquisition services, the successful analysis of a set of performance evaluation samples
- Submittals pertaining to the items or services to be provided must be approved prior to use or initiation of the work on the project site.

I6.1.3 Acceptance of Services

The procurement control will include flow-down provisions of the contract and site-specified task order. The acceptance methods used (e.g., source verification, receipt inspection, and technical verification of data produced) will be verified. Confirmation of specific characteristics

will be performed at intervals and to a depth consistent with the service's complexity, quantity and frequency of procurement, and statement of work requirements.

I6.1.4 Receipt Inspection and Verification

The Site CQCSM or designee will develop and implement procedures for receipt inspection and verification of purchased items. These controls will provide for the following, as applicable:

- Verification that the items received are in accordance with purchase order requirements
- Inspection for evidence of breakage, damage, or other indications of unfitness for use
- Verification that required documentation is received and is acceptable
- Verification that the items conform to the supplier's published requirements that were provided, submitted, and approved.

I6.1.5 Handling, Storage, Packaging, and Shipping

The handling, storage, cleaning, preservation, packaging, and shipping of items shall be controlled to prevent damage or deterioration that would jeopardize the specified performance of the items.

Procurement documents shall include the following:

- Requirements for sellers to establish special procedures, when necessary, to ensure cleanliness, identification, and proper handling
- Requirements for the preparation of items for shipment, as necessary, to prevent damage or deterioration of the supplied items
- Requirements for material and equipment storage instructions, when specified, to be available at the site well in advance of the arrival of material/equipment.

I6.2 Subcontractor Quality Control

All subcontractors performing work for a project are responsible for compliance to the requirements of their respective subcontract. Subcontractors include organizations supplying quality-related items or services to the project. The overall responsibility for conformance to the quality requirements for the subcontracted items and services is retained by APTIM.

The requirements for personnel qualifications, technical performance levels, QC procedures, acceptability levels, and documentation will be included as part of the subcontract documents. The PM or designee will review the subcontract procurement documents to verify that QC requirements are passed on to the subcontractor.

The Site CQCSM is responsible for the implementation of inspections, surveillance, document review, audits, and other QC activities to verify subcontractor compliance with the contract and subcontract requirements. These activities will be documented on inspection reports, audit reports, field logs, or other forms appropriate to the function performed.

For field operations, the field QA/QC personnel will provide QC checks before, during, and at the completion of the subcontractor's activities to determine that the subcontractor is in compliance with the QC measures set forth by the contract, the applicable subcontract documents, and the subcontractor's approved QC plan, including the following:

- Meeting quality requirements
- Generating, controlling, and maintaining required documentation
- Performing and documenting required inspections and tests
- Identifying, reporting, and correcting nonconforming conditions
- Turnover to the contractor.

I6.3 Analytical Laboratory Services

Analytical testing will be performed using analytical laboratories off the project site as specified by the contract documents. The analytical testing requirements and related activities are described in Chapter 4.0 of the CMI work plan.

I6.3.1 Other Subcontractors

Subcontractors performing work other than laboratory-related activities will be monitored by the Site CQCSM or field QA/QC personnel to verify conformance to the contract and subcontract quality requirements. The monitoring activities will include audits, surveillances, witnessing of inspections and tests, document reviews, and interfacing with the subcontractor's QC or project management. All monitoring activities will be documented on the appropriate form or included in the daily construction log.

I6.3.2 Subcontractor Noncompliance

Work performed by subcontractors that does not comply with the specified requirements will be identified, reported, controlled, tracked, and corrected.

I7.0 Audits

Audits may be performed to verify compliance with aspects of the project documents. Audits will be performed with checklists and will include a review of documents and records to determine whether the CQAP and supporting procedures are being implemented. A site-specific assessment checklist for RSA-271 is provided as Attachment 1 of this CQAP. The individual elements of the checklist are based on the site-specific requirements presented in Worksheet No. 14 (Appendix E) and Chapter 4.0 in the CMI work plan.

An audit will note findings and observations. A finding is a documented statement of fact concerning a noncompliance or deviation from established requirements. An observation is a statement of fact regarding the potential for a noncompliance.

Audits will be performed by qualified personnel, including individuals that are technically knowledgeable in the areas to be assessed. Audit results will be documented and sent to the appropriate management.

I7.1 Scheduling and Planning

Audits typically will be performed early in the life of the activity as practical and continue until completion of the activity. The Auditor will provide written notification to the organization to be audited informing them of the scheduled audit date.

Audit schedules may be prioritized based on the importance of the activity, previously identified deficiencies of the activity, and the size or complexity of the activity. The Site CQCSM will develop a schedule for the performance of audits. The audit schedule will be posted and distributed to project staff and managers. Unscheduled audits may be used to supplement scheduled audits when conditions warrant.

I7.2 Internal Performance Audits

Performance audits are conducted on site by an Auditor who directly observes specific project activities to determine if these activities are being conducted in accordance with the contract requirements. The Auditor will be technically competent in the activities to be audited and independent of the subject work. The audit of project deliverables will determine compliance with the procedures set forth in this plan (i.e., technical reviews, documentation of reviews, document control, and other procedures). Checklist items to be examined may include the following:

- Availability and implementation of approved work instructions
- Field documentation and checking
- Subcontractor performance
- Review of personnel training and qualification records
- Review of process controls and associated records to determine compliance with CMI work plan
- Review of work areas for evidence of implementation of procedures and instructions
- Review of documentation indicating compliance with plan, document and design preparation, review, and approval procedures
- Change/nonconformance documentation and disposition.

I7.3 Execution of Audits

Audits will normally be conducted as described in Sections 7.3.1 through 7.3.4.

I7.3.1 Pre-Audit Meeting

The Auditor will conduct a brief pre-audit meeting with management or supervisory personnel of the organization to be audited to confirm the audit scope, discuss the audit sequence, establish a tentative time for the post-audit meeting, and establish channels of communication.

I7.3.2 Audit

The Auditor will follow checklists, developed prior to the audit, to evaluate existing project records provided by designated project staff and may observe work in progress. If noncompliances are observed or uncovered during the audit, the Auditor will discuss these potential findings with the individuals being audited so that findings are accurate and understood. In addition to identifying noncompliances, the audit results may include observations of notable areas of strength.

I7.3.3 Exit Meeting

Upon completion of the audit, the Auditor will discuss observations and findings with the group or organizations audited and, whenever possible, agree on corrective actions. Minor administrative findings that can be resolved to the satisfaction of the audit team during the audit are not required to be documented as items requiring corrective actions. All findings that are not resolved during the course of the audit and findings affecting quality will be noted on the audit checklists.

I7.3.4 Audit Report

The Auditor will prepare and issue an audit report that provides the following information at a minimum:

- Unique audit number
- Description of the audit scope
- Audited organization and location
- Persons contacted during the audit activities
- Audit dates
- Summary of audit results, including a statement on the effectiveness of the quality management elements which were audited
- Suggested opportunities for improvement in the form of observations and comments
- Description of each reported audit finding in sufficient detail to enable corrective action to be performed.
- Due date for completion of corrective actions and/or audit response, typically 30 days.

Audit results will include findings and observations. Findings are items that require corrective action. Findings will be documented on an audit finding report or equivalent. Observations are nonmandatory recommendations to improve project quality. The Auditor may make recommendations for corrective actions; however, the ultimate responsibility for taking corrective action lies with the auditee. The report will be signed by the Auditor. Checklists need not be included with the audit report, but should be maintained as records in the project central files.

The Auditor will prepare an audit report cover letter or memorandum for signature and issuance by the APTIM PM. The audit report will be issued to the management of the audited organization.

I7.4 Response

The response prepared by the auditee will clearly state for each finding the corrective action taken or planned, the cause of the deficiency, and the action to prevent recurrence. For each observation, the response will indicate actions taken or planned for quality improvement. The response will, at a minimum, be sent to the APTIM PM and the Auditor.

I7.5 Follow-Up

The Site CQCSM or designee will track all audit findings to assure that all findings are appropriately addressed and for significant conditions adverse to quality. The Site CQCSM or designee will maintain the status of audit findings for active audits and prepare correspondence relating to overdue audit responses. When responses are overdue, the Site CQCSM or designee notifies the responsible organization by telephone that responses are overdue and prepares a memorandum or letter indicating a new response due date. If a request for extension of response is received, an evaluation will be made and a formal response submitted to the requesting organization.

The APTIM PM or designee, upon receipt of responses to audit findings, will coordinate with the Auditor for the evaluation of responses. The responsible evaluator will document the results of the evaluation. Unacceptable responses will be noted together with the specific reason for rejection. The APTIM PM or designee will prepare transmittal correspondence to the responsible organization to inform them of a new response due date.

Follow-up actions, possibly including re-audit of deficient areas, will be taken to verify whether corrective action is accomplished as scheduled. The Site CQCSM or designee will assure that verifications of corrective action implementation are accomplished and document the results of verification.

Following acceptance and verification of all corrective actions, an audit closure document will be issued by the Auditor to the same distribution as the audit report. The closure document will indicate that corrective actions have been satisfactorily completed and will contain a statement that the audit is closed.

I7.6 Documentation

The following documents generated before, during, and after the audit process will be maintained in the record file system in accordance with Chapter 4.0 of this CQAP.

- Audit report
- Audit responses
- Audit closure letter
- Correspondence related to the audit.

I8.0 Construction Inspections

The primary function of inspections is to establish the measures required to verify the quality of work performed and compliance to the specified requirements, including the inspection of materials and workmanship before, during, and after each work element.

I8.1 Preparatory Inspections and Meetings

Along with representatives of RSA, the PM, Site CQCSM, and Site Supervisor will conduct preparatory inspections/meetings at RSA-271. Preparatory inspections/meetings will be performed prior to starting definable features of work. Typical definable tasks and related inspection requirements can be modified based on project requirements. Where more than one work element is included in one work activity, one preparatory meeting may cover several work elements for the site. Likewise, a number of work activities, where feasible, can be combined into individual preparatory meetings. The preparatory inspection/meeting will be attended by the Army, applicable contractor personnel and subcontractors involved with the feature of work, and responsible field QA/QC personnel. The Site CQCSM will be notified in advance to coordinate participation in the inspection. The preparatory meeting may include but not be limited to the following:

- Review the basic elements of the work.
- Review documentation and reporting requirements.
- Review pertinent contract requirements.
- Review materials and equipment documentation for required tests, submittals, and approvals.
- Review required QC inspections and test requirements.
- Establish that the preliminary work required to begin the feature of work is complete and conforms to approved drawings and submittal data.
- Establish that the required materials and equipment for commencement of the work are on hand or available for use on the feature of work and that all equipment is properly calibrated and in proper working condition.
- Ensure the securing of utility clearances (digging permits).
- Address basic site health and safety considerations.
- Establish hours of operation.

Preparatory inspections will be reported on the daily construction logs. The detailed results of the preparatory inspection will be documented.

Personnel performing work activities affected by a preparatory inspection will be directed in the acceptable level of the workmanship involved for the feature of work covered by the inspection.

I8.2 Initial Inspections

The initial inspection may be conducted at the beginning of the work element. The inspection will be performed as soon as it is determined that a sufficient portion of the work element has been accomplished to evaluate the following criteria:

- Compliance with the CMI work plan, drawings, submittals, and other contract requirements
- Acceptable levels of workmanship
- Quality of materials
- Resolution of differences (when applicable).

Initial inspections will include participation of the responsible personnel, including appropriate subcontractors and the field QA/QC personnel involved with the work element. The Army will be notified in advance of each initial inspection to coordinate participation in the inspection. The initial inspections will be reported on the daily construction logs.

I8.3 Follow-Up Inspections

Follow-up inspections will be performed throughout the course of work. The frequency of the follow-up inspections will depend on the extent of work being performed on each particular work element. Follow-up inspections will also be performed on completed work phase prior to starting subsequent phases. Deficiencies identified will be corrected in a timely manner or identified on a punch list which will be used as a tracking method until the work is completed and verified and the punch list item signed off. Deficiencies which would be made inaccessible for correction by subsequent work activities will be corrected and accepted prior to starting the new work.

The follow-up inspections will be reported on the daily construction logs and copies of the inspection forms as applicable.

I8.4 Pre-Final Inspection

Near the completion of the work or increment thereof as established, the contractor Site CQCSM shall conduct an inspection of the work and develop a punch list of items which do not conform

to the approved CMI work plan. The list of deficiencies will become a part of the CQC documentation, which will include the estimated date by which the deficiencies will be corrected. The contractor Site CQCSM will then make a second inspection to ensure that all deficiencies have been corrected. Once this is completed, the contractor will notify the Site CQCSM that the site is ready for pre-final inspection.

The Army will perform the pre-final inspection to verify that the site work has been satisfactorily completed. A pre-final punch list may be developed by the Site CQCSM as a result of this inspection. The contractor Site CQCSM will then make sure that all items on this list have been corrected and notify the PM so that a final inspection with RSA can be scheduled. Items noted in the pre-final inspection will be completed in a timely manner. These inspections and deficiency corrections will be accomplished within the time frame slated for completion of the project.

I8.5 Final Inspection

The PM, Site Supervisor, Site CQCSM, and Army representatives will be in attendance at this inspection. The RSA representative will formally schedule the final inspection based upon completion of the results of the pre-final inspection. Notice will be given to the Army at least 14 days prior to the final inspection and must include APTIM's assurance that all the specific items previously identified as being unacceptable will be completed by the date scheduled for the final inspection.

APTIM will prepare the punch list. The punch list will identify all nonconforming or incomplete work. Upon completion of the punch list items, a second inspection will be conducted by RSA and APTIM to verify all of the items conform to the requirements. The QA/QC Director will be the final authority to accept that all of the punch list items have been corrected.

I8.6 Inspection Documentation

The Site CQCSM is responsible for the maintenance of the inspection records. Inspection records will be legible and clearly provide all information necessary to verify the items or activities inspected conform to the specified requirements or, in the case of nonconforming conditions, provide evidence that the conditions were brought into conformance or otherwise accepted by the Army.

I9.0 Analytical Testing

The Army's IW QAPP (Shaw, 2013 and updated) or most recent based on submissions to ADEM establishes the measures for management and control of analytical testing activities affecting the quality of the corrective measures. Control of construction (through performance of analytical QC testing) is the responsibility of APTIM (including its subcontractors) and will be in accordance with the CMI work plan.

Task-specific programs of field controls, consisting of inspections and verification tests, will be utilized to verify that the CMI work plan is adhered to during implementation of the corrective measures, where applicable. However, the IW QAPP presents the overall general principles that are employed in any chemical analyses performed as part of the construction. Detailed analytical testing requirements for the RSA-271 CMI are provided in site-specific Uniform Federal Policy worksheets (Appendix E).

I10.0References

Alabama Department of Environmental Management (ADEM), 2020, ***Redstone Arsenal's Alabama Hazardous Wastes Management and Minimization Act Hazardous Waste Storage Facility, Thermal Treatment, Solid Waste Management Unit Corrective Action Permit, Modification No. 15***, 21 September.

CB&I Federal Services LLC (CB&I), 2017, ***Revision 4 RCRA Facility Investigation Report RSA-271, Former Boiler House, Building 7729, Operable Unit 10 Redstone Madison County, Alabama,*** May.

Conestoga-Rovers & Associates, 1996, ***Phase I Environmental Baseline Study, Redstone Arsenal Rocket Engine Facility North Plant, Madison County, Alabama***, prepared for Thiokol Corporation, November.

Shaw Environmental, Inc. (Shaw), 2013, ***Revision 2, Installation-Wide Quality Assurance Program Plan for the Program Management Contract, Volumes I and II***, ***U.S. Army Garrison, Redstone Arsenal, Madison County, Alabama***, May.

Shaw Environmental, Inc. (Shaw), 2005, ***Draft RSA-146 Potential Source Area Investigation, Redstone Arsenal, Madison County, Alabama***, prepared for the U.S. Army Corps of Engineers, Savannah District, Savannah, Georgia, January.

TABLES

Table I.4-1

Typical Daily Construction Log
Redstone Arsenal, Madison County, Alabama

Date: _____

Contractor: _____ Project No.: _____

Contract Title: _____ Report No.: _____

Area: _____ Unit: _____ Work: Contract (✓) _____ WO # _____

Shift: _____ Hrs. Wkd: From _____ To _____ Weather Temp. _____ L H

Manpower	No.	Total Hrs.	Major Equipment	No.	Total Hrs.

Description of Work Performed Today:

Remarks by Contractor: (Delays, interruptions, deviations, extra work activities, unusual occurrences, etc., relevant to today's work.)

For Contractor: Title: Date:

USACE Comments and/or Exceptions:

For USACE: Title: Date:

Table I.5-1

Nonconformance Report
Redstone Arsenal, Madison County, Alabama

Linked w/Variance No: Page __of __

Project Name:	Project Number:
Date of Issue:	Report Number:
-- Nonconformance Report --	
I. Description of the Nonconformance, include requirement violated: (by the person identifying the nonconformance) Identified by: Date:	
Root Cause of nonconformance:	
II. Recommended Corrective Action: (by the person identifying the nonconformance and the review committee)	
To Be Performed by:	Date:
To Be Verified by:	Date:
III. Corrective Action Implementation: (by those implementing the corrective action)	
Was Performed by:	Date:
Was Verified by:	Date:
How was the Corrective Action Verified?	
IV. Nonconformance Resolution, include action taken to preclude recurrence: (by the review committee)	
Affected Organization:	**-- Signatures --**
Distribution List:	Requested by: _____ Date: _____ (printed name and date) Signature:
	QC Approved by: _____ Date: _____ (printed name and date) Signature:
	Proj. Mgr. Approval:_____ Date: _____ (printed name and date) Signature:
	Client QA Approval: _____ Date: _____ (printed name and date) Signature:

Table I.5-2

Nonconformance Report Tracking Register
Redstone Arsenal, Madison County, Alabama

PROJECT NO.
CONTRACT NO.
NONCONFORMANCE REPORT TRACKING REGISTER

NCR NO.	DESCRIPTION OF NONCONFORMANCE	DATE ISSUED	DATE CLOSED	COMMENTS

Table I.5-3

Stop Work Notice
Redstone Arsenal, Madison County, Alabama

Project Name/Location: _____ Project No. _____ D.O. No. _____

S.W.O. No. _____ Date: _____ Page 1 of ___

1. Written Notice Issued to: Name: _____ Title: _____ Org.: _____	2. P.O. # or Activity: _____ 3. Location: _____ 4. Issued by (name): _____ Issued by (title): _____
5. Verbal Notice Issued to: Name: _____ Title: _____	Date: _____ Time: _____
6. Associated NCR No.: _____	7. Associated CAR No.: _____
8. Stop Work Order Condition Description:	Attachment _____
9. Remedial Action Required: By Whom: _____ Required Remedial Action Determined by: Project Manager: _____ CQA Director/Field CQA Coordinator: _____	Attachment _____ By When: _____ Date: _____ Date: _____
10. Follow-up of Remedial Action Taken: Verbal Notice to Resume Operations Given to: Name: _____ Title: _____ Stop Work Order Cancellation Authorized by: CQA Director/Field CQA Coordinator: _____	Attachment _____ Date: _____ Time: _____ Date: _____

Table I.5-4

Stop Work Notice Log
Redstone Arsenal, Madison County, Alabama

SWO No.	Action Party/Organization	Subject	Date Issued	Date Closed

ATTACHMENT 1

FIELD INSPECTION CHECKLIST

	Contract No: W91ZLK-13-D-0018 **EPA ID No.** AL7 210 020 742	**Controlling Document:** **Corrective Measures Implementation (CMI) Plan**
Location: Redstone Arsenal, Madison County, Alabama		
Inspection Type: Field Inspection Checklist **Subject:** RSA-271 CMI Work Plan	**Inspection Date:**	

Descriptor / Requirements	Comments	Results
4.0 Corrective Measures Implementation		
This section provides an overview of the field activities planned to complete corrective measures at RSA-271. Work presented in this section will be completed in accordance with the procedures described in the CMI Work Plan and other approved documentation as appropriate. RSA-271 CMI Work Plan Chapter 4.0		
4.1 General Scope		
The general scope of work (SOW) includes the following: • Mobilization/demobilization • Site control and equipment staging • Utility clearance and marking • Surveying • Monitoring well installation and development • Implementation of site-specific groundwater use restrictions • Baseline groundwater sampling • Monitored natural attenuation and long-term monitoring (groundwater sampling and reporting) • Waste characterization and management of waste generated during groundwater sampling. RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama

Inspection Type: Field Inspection Checklist
Subject: RSA-271 CMI Work Plan

Descriptor / Requirements	Comments	Results
4.1.1 Procurement and Subcontracting		
Subcontracted services and imported materials required for the completion of the project may include the following: • Storm water erosion and sediment controls • Vegetation clearance • Monitoring well installation and development • Surveying • Groundwater sampling • Analytical laboratory services • Remediation-derived waste (RDW)/investigation-derived waste (IDW) transportation and disposal (e.g., sampling purge and decontamination water). Support equipment and materials may be procured through equipment vendors and scientific supply vendors and shipped directly to the site. RSA-271 CMI Work Plan Chapter 4.0		
4.1.2 Field Personnel		
The following field personnel may be utilized to complete field corrective measure activities: • Site supervisor • Site safety officer • Field quality control site manager • Groundwater sampling technician(s) • Equipment operator • Laborer. Drilling and surveying activities will be subcontracted. Groundwater sampling may be performed in house or subcontracted. The number and schedule of personnel will be adjusted during the project as required for completion. RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama

Inspection Type: Field Inspection Checklist
Subject: RSA-271 CMI Work Plan

Descriptor / Requirements	Comments	Results
4.1.3 Quality Control Inspections for Field Activities		
Inspections may be performed and verified through visual observation, measurement of materials or equipment, examination of documentation/certification, evaluation of performance, or testing. RSA-271 CMI Work Plan Chapter 4.0		
Inspections will be performed using a three-phase inspection method: 1) Preparatory inspection(s) are performed prior to start-up and will examine training, procedures, equipment and materials, work plans and documents, and overall readiness to perform work; 2) Initial inspection(s) are performed when work begins on a particular feature of work and include an examination of the quality of workmanship and a review of control testing for compliance with work plan requirements. Follow-up inspection(s) are then performed to verify compliance with procedures; and 3) Follow-up inspections will ensure a continuation of quality and safety standards established during preparatory and initial inspections until completion of the definable work feature. Final follow-up inspection(s) will be conducted at the completion of the activity. RSA-271 CMI Work Plan Chapter 4.0		
4.1.4 Daily Reports		
Daily reports (including daily construction logs, etc.) are provided to the APTIM Project Manager or their designee during CMI activities. RSA-271 CMI Work Plan Chapter 4.0		
4.1.5 Safety and Health Requirements		
All personnel involved in the corrective measures are following this CMI work plan and abide by the health and safety requirements presented in the site-specific safety and health plan. RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama

Inspection Type: Field Inspection Checklist
Subject: RSA-271 CMI Work Plan

Descriptor / Requirements	Comments	Results
4.2 Preliminary Activities		
Preliminary activities include submittal of a job order request, mobilization, surveying, utility marking, obtaining dig permits, and protection of existing site features. In accordance with 40 Code of Federal Regulations 112, a Spill Prevention, Control, and Countermeasures plan will be prepared if fuel storage is required on-site and the aboveground storage capacity of a single container is in excess of 660 gallons or the aggregate aboveground storage capacity is greater than 1,320 gallons. Monitoring well installation may require clearing of vegetation. Storm water erosion and sediment controls will be installed as required by clearing activities associated with monitoring well installation or groundwater sampling activities. All field personnel will follow this work plan and abide by the health and safety requirements presented in a site-specific safety and health plan to be prepared by APTIM. RSA-271 CMI Work Plan Chapter 4.0		
4.2.1 Mobilization		
Mobilization will include deployment of personnel, equipment, subcontractors, and materials necessary to commence CMI activities. All APTIM and subcontractor personnel have completed required training and health and safety requirements. This includes: • Occupational Safety and Health Administration 40-hour Hazardous Waste Operations and Emergency Response and 8-hour updates • Reviewed and signed site-specific health and safety plan • Any additional site or contract specific training or health and safety requirements RSA-271 CMI Work Plan Chapter 4.0		
4.2.2 Access to Redstone Arsenal		
New personnel and subcontractor personnel have registered at the Redstone Arsenal (RSA) Visitors Center at Gate 9. Personnel with prior approval from a host RSA organization will be issued a personnel badge upon presentation of proper identification. Temporary passes may be required for some vehicles. Personnel with current badges may access the RSA via any active gate but should note that gate access hours vary. Commercial trucks must pass through the inspection facilities at either Gate 1 or Gate 9 each time they enter RSA. RSA-271 is in an area with restricted access and is surrounded by a high security fence at the Raytheon Operational Area. RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama		
Inspection Type: Field Inspection Checklist **Subject:** RSA-271 CMI Work Plan		
Descriptor / Requirements	**Comments**	**Results**
4.2.3 Digging Permit and Utility Marking		
A job order request that describes the proposed activities was submitted in advance of any intrusive field work. APTIM has coordinated with the appropriate RSA personnel within 14 days of intrusive activities requesting a work order for a digging permit to ensure that any underground utilities in the proposed well location are properly marked and protected. The procedure requires notification by telephone ([256] 876-9881). RSA-271 CMI Work Plan Chapter 4.0		
4.2.4 Site Control		
APTIM using temporary construction fencing materials, barricades, and warning tape, as necessary, to delineate the site exclusion zone, contamination reduction zone, and site support zone in compliance with the site-specific safety and health plan. RSA-271 CMI Work Plan Chapter 4.0		
4.2.5 Storm Water Erosion and Sediment Controls		
Best management practices (BMP), if needed were used in accordance with Alabama Handbook for Erosion Control, Sediment Control and Stormwater Management on Construction Sites and Urban Areas (Alabama Soil and Water Conservation Committee, 2018). Documented inspections of erosion control devices occur at least weekly and within 72 hours of any qualifying precipitation event. Maintenance of BMPs was performed in accordance with the specifications included in the CMI Work Plan and its attachments and documented in the daily reports. RSA-271 CMI Work Plan Chapter 4.0		
4.2.6 Vegetation Clearing		
Vegetation clearance was performed in accordance with the CMI Work Plan, and in coordination with RSA forester for removal of any commercial quality trees. The remaining trees and brush were cleared using manual and mechanical means (e.g., chainsaw, line trimmer, or heavy equipment). The cleared materials have been mulched for beneficial reuse or disposed at the RSA construction and demolition landfill. RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama

Inspection Type: Field Inspection Checklist
Subject: RSA-271 CMI Work Plan

Descriptor / Requirements	Comments	Results
4.2.7 Monitoring Well Installation and Development		
A new overburden monitoring well (271-RS2994) was installed at the site prior to baseline groundwater sampling. The well was installed and developed in accordance with standard operating project procedures (SOPP) No. 8 and 17. RSA-271 CMI Work Plan Chapter 4.0		
4.2.8 Locating, Marking, and Surveying of Well Locations		
The proposed monitoring well location has been surveyed by a licensed land surveyor. The proposed monitoring well location is shown on Figure 4-1 of the Corrective Measures Implementation Plan. RSA-271 CMI Work Plan Chapter 4.0		
4.3 Groundwater Monitoring		
Collected groundwater samples from four monitoring wells (271-RS2707, -RS2708, -RS2709, and RS2994) for the baseline sampling event. Samples were analyzed for the following analytical parameters following collection using low-flow groundwater sampling methods: • Perchlorate • Volatile organic compounds (VOC) • Semivolatile organic compounds (SVOC) with low level polynuclear aromatic hydrocarbons (PAH) • Explosives • Manganese • Water quality parameters – pH, temperature, specific conductance, dissolved oxygen (DO), and oxidation-reduction potential (ORP) (field measurements). RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama

Inspection Type: Field Inspection Checklist
Subject: RSA-271 CMI Work Plan

Descriptor / Requirements	Comments	Results
4.4 Field Records		
A field geologist or engineer is recording all field activities, specifically the following: • Field activity logs • Field equipment calibration logs • Groundwater level measurements • Sample labels • Sample collection logs • Groundwater purge and sample logs • Sample request for analysis/chain-of-custody logs. RSA-271 CMI Plan Section 4.0		
4.5 RDW/IDW Management		
RDW/ IDW generated during the CMI activities at RSA-271 is expected to include decontamination fluids and disposable personal protective equipment (PPE). RSA-271 CMI Plan Section 4.0		
APTIM personnel are managing waste in accordance with the CMIP and ADEM regulations. RSA-271 CMI Plan Section 4.0		
IDW is containerized, labeled, and stored in accordance with Alabama Administrative Code 335-14 as applicable and as described in the Alabama Environmental Investigation and Remediation Guidance (AEIRG) (ADEM, 2017). IDW PPE will be disposed as normal household trash. Other types of IDW, if generated, will be managed in accordance with Table 2 of Appendix G of ADEM (2017). RSA-271 CMI Plan Section 4.0		
4.6 Equipment Decontamination		
Nondisposable sampling equipment is decontaminated prior to beginning work at the site and prior to the collection of individual samples to prevent cross contamination and maintain the integrity of the environmental samples collected. All sampling equipment will be decontaminated in accordance with procedures specified in the RSA installation-wide quality assurance program plan (QAPP) (Shaw, 2013 and as updated), which was prepared in accordance with Appendix E of the AEIRG (ADEM, 2017). RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama		
Inspection Type: Field Inspection Checklist **Subject:** RSA-271 CMI Work Plan		
Descriptor / Requirements	**Comments**	**Results**
4.7 Demobilization		
Personnel, equipment, and subcontractors were demobilized from the project site after completion of remedial activities. RSA-271 CMI Work Plan Chapter 4.0		
4.8 Land-Use Controls		
Land-use controls (LUC) for groundwater implemented as part of RSA's Installation-wide groundwater interim record of decision (IROD) will be continued until final remedies are selected for groundwater site RSA-146. The controls specified as part of the Installation-wide groundwater IROD will continue to protect potential receptors until cleanup goals (CG) in groundwater are attained for chemicals of concern (COC) requiring action at RSA-271. These LUCs prevent potable use of groundwater and require management of nonpotable uses such that exposure to contaminated groundwater is minimized or prevented. The Army's site access control (SAC) program is used to implement the groundwater LUCs. Via the SAC program, the installation of wells for drinking water will be prevented, and all requests for the installation of wells for industrial processes or agricultural purposes for RSA-271 and all other locations on RSA will be subject to Army review and approval. An environmental use restriction (EUR) will be required until CGs are met for groundwater. A draft notice of EUR (NEUR) will be submitted in conjunction with the corrective measures report for this site. LUC monitoring will be performed at this site with details to be specified in the corrective measures report once actions are completed. RSA-271 CMI Plan Section 4.0		
4.9 Corrective Measures Reporting		
RSA will provide a CMI report following the baseline groundwater sampling event. Additional reports (e.g., CMI effectiveness reports) will be prepared documenting groundwater monitoring results, corrective actions, and LUC inspections. RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama

Inspection Type: Field Inspection Checklist
Subject: RSA-271 CMI Work Plan

Descriptor / Requirements	Comments	Results
4.10 Post-Construction Scheduled Activities		
Post-construction scheduled activities include groundwater monitoring and data evaluation, inspections, and reporting. RSA-271 CMI Work Plan Chapter 4.0		
4.10.1 Groundwater Monitoring		
Annual groundwater monitoring was conducted until CGs have been attained for the groundwater COCs for three consecutive years. Groundwater samples were collected using low-flow purging and sampling techniques. The groundwater samples were analyzed for the same analytes and field parameters as the baseline sampling event. Groundwater data collected was included in annual reports. RSA-271 CMI Work Plan Chapter 4.0		
4.10.2 Data Evaluation and Interpretation		
Data collected was evaluated to determine whether the corrective measures at RSA-271 reduced COC concentrations in groundwater beneath the site to CGs. Statistical methods used to evaluate groundwater data followed Alabama Administrative Code 335-114-5-.06(8)(h) and EPA guidance for groundwater monitoring at Resource Conservation and Recovery Act (RCRA) facilities (EPA, 1989, 1992, 2009). Non-statistical methods that may also be used to characterize RSA-271 groundwater conditions include hydrographs, potentiometric surface maps, concentration-versus-time plots and plume maps. RSA-271 CMI Work Plan Chapter 4.0		

Location: Redstone Arsenal, Madison County, Alabama		
Inspection Type: Field Inspection Checklist **Subject:** RSA-271 CMI Work Plan		
Descriptor / Requirements	**Comments**	**Results**
4.10.3 Inspections		
General site inspections of RSA-271 were conducted annually to verify that LUCs are being properly implemented per the environmental restriction. A site inspection of RSA-271 was performed to verify if any new wells have been installed and that wells are being used in accordance with the army SAC program. ADEM was notified within 10 days if new wells were discovered during inspection or if any wells were being used in a manner inconsistent with the Army SAC program. LUC inspections were documented in the CMI effectiveness reports. RSA-271 CMI Work Plan Chapter 4.0		
4.10.4 Reporting		
RSA provided CMI effectiveness reports on an annual basis. The initial CMI effectiveness report included all the groundwater monitoring data collected during the first year of the LTM at RSA-271. Subsequent reports included the results of the annual groundwater monitoring events along with the results from any other groundwater monitoring conducted as part of the RSA-271 CMI during that period. Recommendations for changes to the sampling frequency, wells sampled, and parameters for analysis, if appropriate, were included in the reports. These reports documented the groundwater monitoring results and the status of the LUCs. RSA-271 CMI Work Plan Chapter 4.0		

References:

Alabama Department of Environmental Management (ADEM), 2017, ***Alabama Environmental Investigation and Remediation Guidance, Revision 4.0***, February.

Shaw Environmental, Inc. (Shaw), 2013a, ***Installation-Wide Quality Assurance Program Plan for the Program Management Contract, Revision 2, U.S. Army Garrison, Redstone Arsenal, Madison County, Alabama***, May.

U.S. Environmental Protection Agency (EPA), 2009, ***Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities: Unified Guidance***, EPA/530/R-09/007, Office of Resource Conservation and Recovery Program Implementation and Information Division, March.

U.S. Environmental Protection Agency (EPA), 1998, ***Technical Protocol for Evaluating Natural Attenuation of Chlorinated Solvents in Ground Water***, EPA/600/R-98/128, Office of Research and Development, Washington, D.C., September.

U.S. Environmental Protection Agency (EPA), 1992, ***Statistical Analysis of Groundwater Monitoring Data at RCRA Facilities, Addendum to Interim Guidance***, Office of Resource Conservation and Recovery, July.

Location: Redstone Arsenal, Madison County, Alabama		
Inspection Type: Field Inspection Checklist **Subject:** RSA-271 CMI Work Plan		
Descriptor / Requirements	**Comments**	**Results**

U.S. Environmental Protection Agency (EPA), 1989, ***Statistical Analysis of Ground-Water Monitoring Data at RCRA Facilities, Interim Final Guidance***, Office of Solid Waste, Waste Management Division, EPA/530/SW-89/026, July.

Results:
S = Satisfactory
I = Incomplete at the time of audit or surveillance. To be verified at a later date
O = Observation
F = Finding
NA = Not Applicable

References

Alabama Soil and Water Conservation Committee (ASWCC), 2018, ***Alabama Handbook for Erosion Control, Sediment Control and Stormwater Management on Construction Sites and Urban Areas***, July.

Alabama Department of Environmental Management (ADEM), 2017, ***Alabama Environmental Investigation and Remediation Guidance, Revision 4***, February.

Shaw Environmental & Infrastructure, Inc. (Shaw), 2013, ***Revision 2, Installation-Wide Quality Assurance Program Plan for the Program Management Contract, U.S. Army Garrison-Redstone, Madison County***, Alabama, May.

Comments:_____

QA Manager:_____ **Date:**_____

APPENDIX J

ALABAMA BEST MANAGEMENT PRACTICES

Appendix J

Alabama Best Management Practices
RSA-014S, Unlined Inactive Burn Trenches, Unit #2
Operable Unit 14
U.S. Army Garrison-Redstone
Madison County, Alabama
EPA ID No. AL7 210 020 742

Prepared for:

U.S. Army Engineering and Support Center
Huntsville Engineering and Support Center
ATTN: CEHNC-OEC
5021 Bradford Drive East
Huntsville, AL 35805

Prepared by:

Aptim Federal Services, LLC
11400 Parkside Drive, Suite 400
Knoxville, Tennessee 37934

Contract No. W912DY-17-D-0003
Delivery Order No. W912DY19F1116

November 2021

Reference:

Alabama Handbook for Erosion Control, Sediment Control and Stormwater Management on Construction Sites and Urban Areas. Auburn, AL, July 2018.

Site Preparation

A. Construction Exit Pad (CEP)

B. Land Grading (LG)

Surface Stabilization

C. Permanent Seeding (PS)

D. Temporary Seeding (TS)

E. Preservation of Vegetation (PV)

Runoff Conveyance

F. Check Dam (CD)

G. Diversion (DV)

Sediment Control

H. Sediment Barrier (SB)

Other

I. Corrective Action Log

J. CBMPP Amendment Log

K. Grading and Stabilization Log

L. Rainfall Documentation Log

A. Construction Exit Pad (CEP)

Construction Exit Pad (CEP)

Practice Description

A construction pad is a stone base pad or manufactured product designed to provide a buffer area where mud and caked soil can be removed from the tires of construction vehicles to avoid transporting it onto public roads. This practice applies anywhere traffic will be leaving a construction site and moving directly onto a public road or street.

Planning Considerations

Roads and streets adjacent to construction sites should be kept clean for the general safety and welfare of the public. A construction exit pad (Figure CEP-1) should be provided where mud can be removed from construction vehicle tires before they enter a public road.

Where possible the construction exit pad should be located and constructed at a site where surface runoff from the pad will not transport sediment from the pad off the site. If the pad slope toward the road exceeds 2%, a diversion ridge 6” to 8” high with 3:1 side slopes should be constructed across the foundation approximately 15 feet from the entrance. This diversion ridge should divert surface runoff from the pad away from the road and into a sediment trap or basin.

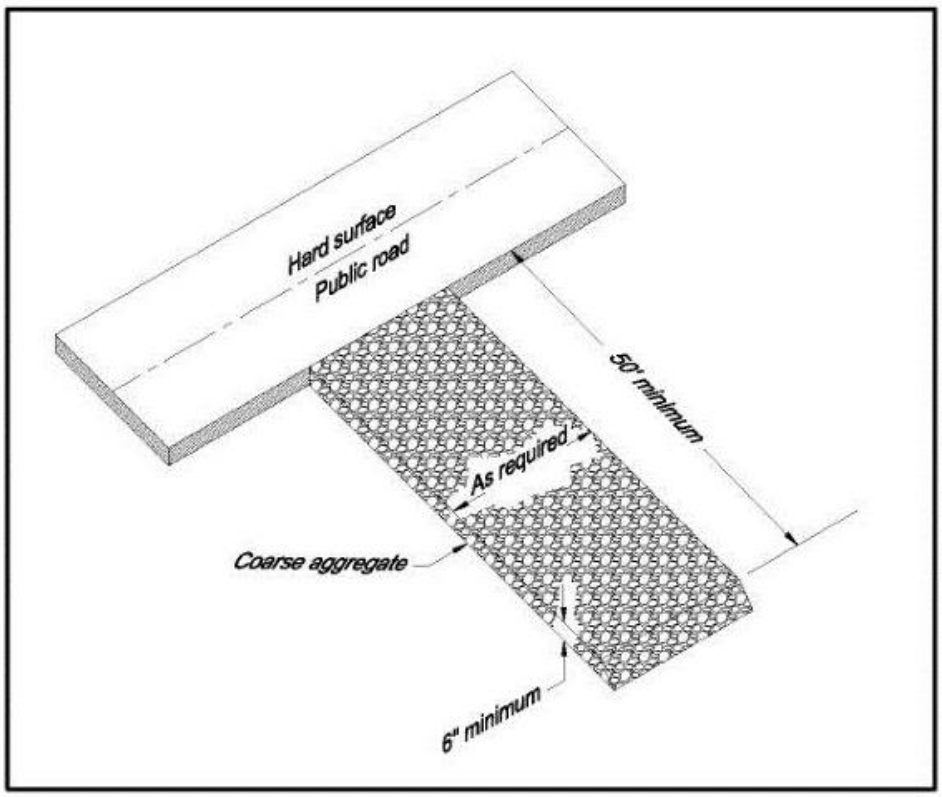


Figure CEP-1 Gravel Construction Exit

If the action of the vehicle traveling over the gravel pad does not sufficiently remove the mud or if the site is in a particularly sensitive area, a washing facility should be included with the pad (Figure CEP-2). When a washing facility is required all wash water shall be diverted to a sediment trap or basin.

If the construction exit pad is located in an area with soils that will not support traffic when wet, an underliner of geotextile will be required to provide stability to the pad.

Construction of stabilized roads throughout the development site should be considered to lessen the amount of mud transported by vehicular traffic. The construction exit pad should be located to provide for maximum use by construction vehicles.

Consideration should be given to limiting construction vehicles to only one ingress and egress point. Measures may be necessary to make existing traffic use the construction exit pad.

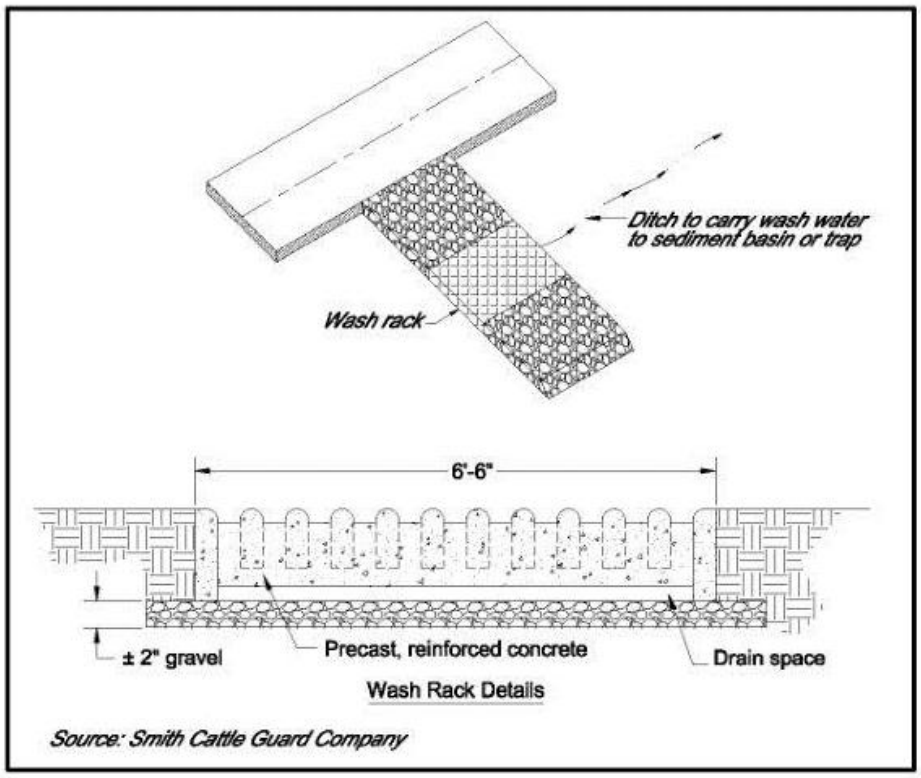


Figure CEP-2 Construction Exit with Wash Rack

Design Criteria

Aggregate size

Aggregate should be Alabama Highway Department coarse aggregate gradation No.1.

Pad Thickness

The exit pad shall have a minimum aggregate thickness of 6”.

Geotextiles

A non-woven geotextile shall be placed underneath the aggregate. The geotextile shall be of the strength and durability required for the project to ensure the aggregate and soil base are stable. Generally, the non-woven geotextile should meet the requirements for a Class 2 geotextile used for separation that is found in the current version of AASHTO M288.

Pad Length

The exit pad should provide for entering and parking the longest anticipated construction vehicles. A pad is typically 50 feet long but the required length may be longer or shorter.

Pad Width

The exit pad width is typically 20 feet but may be narrower or wider to equal the full width of the vehicular egress.

Washing

A washing facility shall be provided if necessary to prevent mud and caked soil from being transported to public streets and highways. It shall be constructed of concrete, stone, and/or other durable materials. Provisions shall be provided for the mud and other material to be carried away from the washing facility to a sediment trap or basin to allow for settlement of the sediment from the runoff before it is released from the site.

B. Land Grading (LG)

Land Grading (LG)

Practice Description

Land grading is reshaping of the ground surface to provide suitable topography for buildings, facilities and other land uses, to control surface runoff, and to minimize soil erosion and sedimentation both during and after construction. This practice applies to sites where the existing topography must be modified to prepare for another land use, or where adapting proposed development to the existing landscape can reduce the erosion potential of the site and the cost of installing erosion and sediment control measures. In some instances, other practices such as diversions or benches can be used to reduce the length of continuous slopes and reduce erosion potential.

Planning Considerations

A detailed plan should be developed by a qualified design professional for all land grading activities at the project site. The plan should show all areas to be disturbed, the areas of cut, areas of fill, and the finished elevation for all graded areas. Areas that will be mowed after the site is developed should have slopes planned that are not too steep for the type of mowing equipment that will be used for regular maintenance.

The grading plan should be designed to protect existing vegetation where possible, especially around natural drainageways. Grading activities should be scheduled to minimize the area disturbed at any one time during the construction process. The plan should include provisions for stabilizing disturbed areas immediately after final grading is completed. Provisions should also be made to protect existing

underground utilities. Finally, topsoil should be removed and stockpiled for use in revegetating the site.

The grading plan should also include necessary practices for controlling sediment and erosion at the site. These practices could include stable outlets and slope breaks such as diversions or benches.

Design Criteria

Site Preparation

A detailed survey of the construction site should be performed by a qualified surveyor prior to grading plan development. This survey should include existing topographic information at the site including existing elevations, existing drainage patterns, locations of existing overhead and underground utilities, and construction limit boundaries.

The grading plan should require that the existing topsoil at sites to be graded be removed as the first step in the grading process. The plan should include a location on the construction site where topsoil will be stockpiled. Stockpiled topsoil should be protected by temporary vegetation (see Temporary Vegetation practice) or other appropriate temporary cover, such as plastic, until it is used to cover disturbed areas in advance of permanent vegetation of the site.

The grading plan should include a schedule of disturbance activities that minimizes the area disturbed at any point in time using sequencing and staging concepts. In areas where clearing of existing vegetation is planned, the area should be cleared and grubbed by removing trees, vegetation, roots and other debris such as trash. In areas to be filled all loose or weak soil and oversized rocks should be removed from the area. The foundation of the area to be filled should consist of soil or rock material of adequate strength to support the proposed fill material and the structures to be built at the site. The exact depth of material to be removed should be determined by a qualified geotechnical professional according to accepted engineering standards.

Grading

A plan for placement of fill should be developed by a qualified geotechnical professional. The plan should specify the source of fill materials, which should be obtained on site if possible. Materials used for fill, when placed according to the plans and specifications, should provide sufficient strength to support structures planned for construction at the location.

Loose fill material should be placed in layers not exceeding 9” in thickness. The materials should be compacted to a moisture content and to a dry density that will produce the design bearing strength required for structures planned at the site. A qualified geotechnical engineer should provide fill placement specifications using standard accepted engineering practices.
Long and/or steep slope lengths can result in rill and gully erosion on slopes. Erosion on these type slopes can be minimized by breaking the slope with

diversions or benches (see Diversion practice). Diversion widths should be compatible with the expected maintenance equipment. Care is needed in locating outlets that will be stable and not cause gully erosion. The following table gives general guidance on the horizontal spacing of slope breaks:

Table LG-1 Guidelines for Spacing Slope Breaks [1]

Slope (H:V)	Horizontal Spacing (Ft)
1:1	20
2:1	40
3:1	60
4:1 and 5:1	80
6:1 to 9:1	120
10:1 or flatter	200

[1] Adjustments in spacing may be made to account for soil and site conditions and professional experience of the site designer.

In areas where seepage and ground water are present subsurface drains should be installed to improve slope stability or soil bearing capacity (see Subsurface Drain practice).

Steep slopes should be avoided if possible. Slopes that are to be vegetated should be 2 horizontal to 1 vertical or flatter. If the slope is to be maintained by tractor or other equipment the slope should be 3 horizontal to 1 vertical or flatter. Slopes should be designed to blend with surrounding topography as much as possible.

Erosion Control

The grading plan should include provisions for stabilization of graded areas immediately after final grading is completed. On areas that will have no additional disturbance, permanent vegetation should be applied immediately to the site (see Permanent Seeding practice) if grading is finished during the planting season. If grading is finished outside of the recommended planting dates a temporary cover should be installed using a Temporary Seeding or other appropriate cover and the Permanent Seeding planned for the next planting period. On areas where work is to be interrupted or delayed for 14 calendar days or longer, such as topsoil stockpiles, the area should be stabilized using mulch or temporary seeding (see Mulching or Temporary Seeding practice). Other stabilization measures such as hydraulic mulch or erosion control blankets should be used in extreme conditions, such as steep slopes and channels.

Where practical, runoff from undisturbed off-site areas should be diverted around the construction site to prevent erosion on the disturbed areas (see Diversion practice).

Sediment Control

Required sediment control practices should be installed before the land disturbance activities in the drainage area of the sediment control practices. Until disturbed

areas can be stabilized, appropriate sediment control measures will be maintained to minimize sediment delivery off-site. Measures should include as a minimum:

- Sediment Barriers – Placed along toes of slopes (see Sediment Barrier practice).

- Sediment Basins – Divert sediment laden runoff to basins as needed to minimize off-site sedimentation (see Sediment Basin practice).

- Inlet Protection – Where sediment-laden runoff is diverted to on-site stormwater drain inlets, the inlets should be protected with an appropriate sediment control practice.

- Stabilized Outlets – All runoff from the site should be conveyed in stabilized channels (see Grassed Swale, Lined Swale, Rip-rap Lined Swale, or other appropriate channel stabilization).

C. Permanent Seeding (PS)

Permanent Seeding (PS)

Practice Description

Permanent seeding is the establishment of perennial vegetation on disturbed areas from seed. Permanent vegetation provides economical long-term erosion control and helps prevent sediment from leaving the site. This practice is used when vegetation is desired and appropriate to permanently stabilize the soil.

Planning Considerations

The advantages of seeding over other means of establishing plants include the smaller initial cost, lower labor input, and greater flexibility of method.

Disadvantages of seeding include potential for erosion during the establishment stage, seasonal limitations on suitable seeding dates, and weather-related problems such as droughts.

The probability of successful plant establishment can be maximized through good planning. The selection of plants for permanent vegetation must be site specific. Factors that should be considered are type of soils, climate, establishment rate, and management requirements of the vegetation. Other factors that may be important are wear, mowing tolerance, and salt tolerance of vegetation.

Plant selection for permanent vegetation should be based on plant characteristics, site and soil conditions, time of year of planting, method of planting, and the intended use of the vegetated area. Climate factors can vary widely in Alabama. Important plant attributes are discussed in Vegetation Establishment for Erosion and Sediment Control in Chapter 2.

Plant selection may include companion plants to provide quick cover on difficult sites, late seedings, or where the desired permanent cover may be slow to establish. Annuals are usually used for companion plants and should be selected carefully to prevent using a species that provide so much competition that it prevents the establishment of the desired species.

Seeding properly carried out within the optimum dates has a higher probability of success. It is also possible to have satisfactory establishment when seeding outside these dates. However, as plantings are deviated from the optimum dates, the probability of failure increases rapidly. Seeding dates should be taken into account in scheduling land-disturbing activities.

Site quality impacts both short-term and long-term plant success. Sites that have compacted soils, soils that are shallow to rock or have textures that are too clayey or too sandy should be modified whenever practical to improve the potential for plant growth and long-term cover success.

The operation of equipment is restricted on slopes steeper than 3:1, severely limiting the quality of the seedbed that can be prepared. Provisions for establishment of vegetation on steep slopes can be made during final grading. In construction of fill slopes, for example, the last 4-6” might not be compacted. A loose, rough seedbed with irregularities that hold seeds and lime and fertilizer is essential for hydroseeding. Cut slopes should be roughened (see Land Grading practice).

Proper mulching is critical to protect against erosion on steep slopes. When using straw, anchor with netting or asphalt. On slopes steeper than 2:1, jute, excelsior, or synthetic matting may be required.

The use of irrigation (temporary or permanent) will greatly improve the success of vegetation establishment.

Design Criteria

Plant Selection

Select plants that can be expected to meet planting objectives. To simplify plant selection, use Figure PS-1 Geographical Areas for Species Adaptation and Seeding Dates and Table PS-1, Commonly Used Plants for Permanent Cover. Mixtures commonly specified by the Alabama Department of Transportation are an appropriate alternative for plantings on rights-of-ways. Additional information related to plants commonly used in Alabama is found in Chapter 2 under the section Vegetation for Erosion and Sediment Control.

The plants used for temporary vegetation may be used for companion plants provided the seeding rate of the annual species is reduced by one half. See the Temporary Seeding practice for additional information on establishing temporary vegetation. **Ryegrass or other highly competitive plants should not be used as a companion plant with a permanent seeding**.

Figure PS-1 Geographical Areas for Species Adaptation and Seeding Dates

Note: Site conditions related to soils and aspect in counties adjacent to or close to county boundaries may justify adjustments in planting dates by qualified design professionals.

Table PS-1 Commonly Used Plants for Permanent Cover with Seeding Rates and Dates

Species	Seeding Rates/Ac PLS	North	Central	South
		Seeding Dates		
Bahiagrass, Pensacola	40 lbs	--	Mar 1-July 1	Feb 1-Nov 1
Bermudagrass, Common	10 lbs	Apr 1-July 1	Mar 15-July 15	Mar 1-July 15
Bahiagrass, Pensacola Bermudagrass, Common	30 lbs 5 lbs	--	Mar 1-July 1	Mar 1-July 15
Bermudagrass, Hybrid (Lawn Types)	Solid Sod	Anytime	Anytime	Anytime
Bermudagrass, Hybrid (Lawn Types)	Sprigs 1/sq ft	Mar 1-Aug 1	Mar 1-Aug 1	Feb 15-Sep 1
Fescue, Tall	40-50 lbs	Sep 1-Nov 1	Sep 1-Nov 1	--
Sericea	40-60 lbs	Mar 15-July 15	Mar 1-July 15	Feb 15-July 15
Sericea & Common Bermudagrass	40lbs 10 lbs	Mar 15-July 15	Mar 1-July 15	Feb 15-July 15
Switchgrass, Alamo	4 Lbs	Apr 1-Jun 15	Mar 15-Jun 15	Mar 15-Jun15

PLS means pure live seed and is used to adjust seeding rates. For example, to plant 10 lbs PLS of a species with germination of 80% and purity of 90%, PLS= 0.8X 0.9 = 72%. 10 lbs PLS = 10/0.72 = 13.9 lbs of the species to be planted.

Seedbed Requirements

Establishment of vegetation should not be attempted on sites that are unsuitable due to compaction or inappropriate soil texture, poor drainage, concentrated overland flow, or steepness of slope until measures have been completed to correct these problems. To maintain a good stand of vegetation, the soil must meet certain minimum requirements as a growth medium. A good growth medium should have these attributes:

- Sufficient pore space to permit root penetration.
- Enough fine-grained soil material (silt and clay) to maintain adequate moisture and nutrient supply.
- Sufficient depth of soil to provide an adequate root zone. The depth to rock or impermeable layers such as hardpans should be 12” or more, except on slopes steeper than 2:1 where topsoiling is not feasible.
- A favorable pH range for plant growth, usually 6.0-6.5.

- Sufficient nutrients (nitrogen, phosphorus and potassium) for initial plant establishment.

- Freedom from large roots, branches, stones, or large clods. Clods and stones may be left on slopes steeper than 3:1 if they are to be hydroseeded.

If any of the above attributes are not met: i.e., if the existing soil is too dense, coarse, shallow or acidic to foster vegetation – chiseling, topsoil, or special amendments should be used to improve soil conditions. The soil conditioners described below may be beneficial or topsoil may be applied (for guidance on topsoiling see Topsoiling practice). These amendments should only be necessary where soils have limitations that make them poor for plant growth or for turf establishment.

- Peat-appropriate types are sphagnum moss peat, reed-sedge peat, or peat humus, all from fresh-water sources. Peat should be shredded and conditioned in storage piles for at least 6 months after excavation.

- Sand-should be clean and free of toxic materials.

- Vermiculite-use horticultural grade.

- Rotted manure-use stable or cattle manure not containing undue amounts of straw or other bedding materials.

- Thoroughly rotted sawdust-should be free of stones and debris. Add 6 lbs of nitrogen to each cubic yard.

Soil Amendments

Liming Materials

Lime (Agricultural limestone) should have a neutralizing value of not less than 90 percent calcium carbonate equivalent and 90 percent will pass through a 10-mesh sieve and 50 percent will pass through a 60-mesh sieve.

Selma chalk should have a neutralizing value of not less than 80 percent calcium carbonate equivalent and 90 percent will pass through a 10-mesh sieve.

Other liming materials that may be selected should be provided in amounts that provide equal value to the criteria listed for agricultural lime or be used in combination with agricultural limestone or Selma chalk to provide equivalent values to agricultural limestone.

Plant Nutrients

Commercial grade fertilizers that comply with current Alabama Fertilizer Laws should be used to supply nutrients required to establish vegetation.

Lime and fertilizer needs should be determined by soil tests. Soil testing is performed by the Auburn University Soil Testing Laboratory and provides recommendations based on field tests on Alabama soils. The local county Cooperative Extension Service can provide information on obtaining soil tests. Commercial laboratories that make recommendations based on soil analysis may be used.

When soil tests are not available, use the following rates for application of soil amendments.

Sandy soils: Use 1 ton/acre (exception on sandy soils – if the cover will be tall fescue and clover) use 2 tons/acre.
Clayey soils: 2 tons/acre.
(Do not apply lime to alkaline soils).

Grasses alone: Use 400 lbs/acre of 8-24-24 or the equivalent. Apply 30 lbs of additional nitrogen when grass has emerged and begun growth (approximately 0.8lbs/1000 ft^2).
Grass-legume mixtures: Use 800 to 1200 lbs/acre of 5-10-10 or the equivalent.
Legumes Alone: Use 400 to 600 lbs/acre of 0-20-20 or the equivalent.

Note: Fertilizer can be blended to meet exact fertilizer recommendations. Take soil test recommendations to local fertilizer dealer for bulk fertilizer blends. This may be more economical than bagged fertilizer.

Application of Soil Amendments

Apply lime and fertilizer evenly and incorporate into the top 6” of soil by disking, chiseling or other suitable means during seedbed preparation. Operate machinery on the contour. On sites too steep for seedbed preparation, fertilizer and lime can be applied with a hydroseeder.

Seedbed Preparation

If needed, grade and shape to provide a surface on which equipment can safely and efficiently be used for seedbed preparation and seeding.

Install necessary sediment control practices before seedbed preparation and complete grading according to the approved plan.

Prepare a friable seedbed with tillage to a depth of at least 6”. Break up large clods, alleviate compaction, and smooth and firm the soil into a uniform surface. Fill in or level depressions that can collect water.

Planting Methods

Seeding

Use certified seed for permanent seeding whenever possible. Certified seed is inspected by the Alabama Crop Improvement Association to meet high quality standards and will be tagged with a “Certified Seed” tag. (Note: all seed sold in

Alabama is required by law to be tagged to identify seed purity, germination, and presence of weed seeds. Seed must meet state standards for content of noxious weeds.)

Seeding dates are determined using Figure PS-1 and Table PS-1.

Inoculate legume seed with the Rhizobium bacteria appropriate to the species of legume. Details of legume inoculation are located in Chapter 2 in the part on Vegetation for Erosion and Sediment Control under Inoculation of Legumes.

Plant seed uniformly with a cyclone seeder, a drill seeder, a cultipacker seeder, or by hand on a fresh, firm, friable seedbed. If the seedbed has been sealed by rainfall, it should be disked so the seed will be sown into a freshly prepared seedbed.

When using broadcast-seeding methods, subdivide the area into workable sections and determine the amount of seed needed for each section. Apply one-half the seed while moving back and forth across the area, making a uniform pattern; then apply the second half in the same way, but moving at right angles to the first pass.

Cover broadcast seed by raking or chain dragging; then firm the surface with a roller or cultipacker to provide good seed contact. Small grains should be planted no more than 1" deep and grasses and legume seed no more than ½" deep.

Hydroseeding

Surface roughening is particularly important when hydroseeding, as a roughened slope will provide some natural coverage for lime, fertilizer, and seed. The surface should not be compacted or smooth. Fine seedbed preparation is not necessary for hydroseeding operations; large clods, stones, and irregularities provide cavities in which seeds can lodge.

Mix seed, inoculant if required, and a seed carrier with water and apply as a slurry uniformly over the area to be treated. The seed carrier should be a cellulose fiber, natural wood fiber or other approved fiber mulch material which is dyed an appropriate color to facilitate uniform application of seed. Use the correct legume inoculant at 4 times the recommended rate when adding inoculant to a hydroseeder slurry. The mixture should be applied within one hour after mixing to reduce damage to seed.

Fertilizer should not be mixed with the seed-inoculant mixture because fertilizer salts may damage seed and reduce germination and seedling vigor.

Fertilizer may be applied with a hydroseeder as a separate operation after seedlings are established.

Lime is not normally applied with a hydraulic seeder because it is abrasive but if necessary it can be added to the seed slurry and applied at seeding or it may be applied with the fertilizer mixture. Also, lime can be blown onto steeper slopes in dry form.

Sprigging

Hybrid bermudagrass cannot be grown from seed and must be planted vegetatively. Vegetative methods of establishing common and hybrid bermudagrass, centipedegrass and zoysia include sodding, plugging and sprigging (see Sodding practice).

When sprigs are planted with a sprigging machine, furrows should be 4-6" deep and 2 feet apart. Place sprigs no farther than 2 feet apart in the row and so that at least one rooting node is in the furrow.

When broadcasting is used for sprig planting, broadcast sprigs at the specified rate (Table PS-1). Press into the top ½" to 2" of soil with a cultipacker or with a disk set nearly straight so that the sprigs are not brought back to the surface. A mulch tacking machine may be used to press sprigs into the soil.

Mulching

The use of mulch provides instant cover and helps ensure establishment of vegetation under normal conditions and is essential to seeding success under harsh site conditions (see Mulching practice). Harsh site conditions include: slopes steeper than 3:1 and adverse soils (shallow, rocky, or high in clay or sand). Areas with concentrated flow should be treated differently and require sod, a hydromulch formulated for channels or an appropriate erosion control blanket.

Irrigation

Moisture is essential for seed germination and vegetation establishment. Supplemental irrigation can be very helpful in assuring adequate stands in dry seasons or to speed development of full cover. It is a requirement for establishment of vegetation from sod and sprigs and should be used elsewhere when feasible. However, irrigation is rarely critical for low-maintenance vegetation planted at the appropriate time of the year.

Water application rates must be carefully controlled to prevent runoff. Inadequate or excessive amounts of water can be more harmful than no supplemental water.

Maintenance

Generally, a stand of vegetation cannot be determined to be fully established until soil cover has been maintained for 1 full year from planting. Inspect vegetated areas for failure and make necessary repairs and vegetate as soon as possible.

If a stand has inadequate cover, reevaluate choice of plant materials and quantities of lime and fertilizer. Re-establish the stand after seedbed preparation or over-seed the stand. Consider a temporary seeding if the time of year is not appropriate for establishment of permanent vegetation (see Temporary Seeding practice).

If vegetation fails to grow, a soil test should be made to determine if soil acidity or nutrient imbalance is responsible.

To attain complete establishment, fertilization is usually required in the second growing season. Turf grasses require annual maintenance fertilization. Use soil tests if possible or follow the guidelines given for the specific seeding mixtures.

Protect vegetation during its establishing period from traffic that will be harmful. If appropriate, use either temporary fences or barriers to protect areas that may be damaged by excessive traffic.

D. Temporary Seeding (TS)

Temporary Seeding (TS)

Practice Description

Temporary seeding is the establishment of fast-growing annual vegetation from seed on disturbed areas. Temporary vegetation provides economical erosion control for up to a year and reduces the amount of sediment moving off the site.

This practice applies where short-lived vegetation can be established before final grading or in a season not suitable for planting the desired permanent species. It helps prevent costly maintenance operations on other practices such as sediment basins and sediment barriers. In addition, it reduces problems of mud and dust production from bare soil surfaces during construction. Temporary or permanent seeding is necessary to protect earthen structures such as dikes, diversions, grass-lined channels and the banks and dams of sediment basins.

Planning Considerations

Temporary vegetative cover can provide significant short-term erosion and sediment reduction before establishing perennial vegetation.

Temporary vegetation will reduce the amount of maintenance associated with sediment basins.

Temporary vegetation is used to provide cover for no more than 1 year. Permanent vegetation should be established at the proper planting time for permanent vegetative cover.

Certain plants species used for temporary vegetation will produce large quantities of residue which can provide mulch for establishment of the permanent vegetation.

Proper seedbed preparation and selection of appropriate species are important with this practice. Failure to follow establishment guidelines and recommendations carefully may result in an inadequate or short-lived stand of vegetation that will not control erosion.

The selection of plants for temporary vegetation must be site specific. Factors that should be considered are type of soils, climate, establishment rate, and management requirements of the vegetation. Other factors that may be important are wear, mowing tolerance, and salt tolerance of vegetation.

Seeding properly carried out within the optimum dates has a higher probability of success. It is also possible to have satisfactory establishment when seeding outside these dates. However, as plantings are deviated from the optimum dates, the probability of failure increases rapidly. Seeding dates should be taken into account in scheduling land-disturbing activities.

Site quality impacts both short-term and long-term plant success. Sites that have compacted soils should be modified whenever practical to improve the potential for plant growth.

The operation of equipment is restricted on slopes steeper than 3:1, severely limiting the quality of the seedbed that can be prepared. Provisions for establishment of vegetation on steep slopes can be made during final grading. In construction of fill slopes, for example, the last 4-6” might not be compacted. A loose, rough seedbed with irregularities that hold seeds and fertilizer is essential for hydroseeding. Cut slopes should be roughened (see practice Land Grading).

Good mulching practices are critical to protect against erosion on steep slopes. When using straw, anchor with netting or asphalt. On slopes steeper than 2:1, either hydraulic mulch or erosion control blanket is more appropriate than straw to protect the slope.

The use of irrigation (temporary or permanent) will greatly improve the success of vegetation establishment.

Design Criteria

Plant Selection

Select plants that can be expected to meet planting objectives. To simplify plant selection, use Table TS-1, Commonly Used Plants for Temporary Cover and Figure TS-1, Geographical Areas for Species Adaptation and Seeding Dates. Seeding mixtures commonly specified by the Alabama Department of Transportation are an appropriate alternative for plantings on rights-of-ways. Additional information related to plantings in Alabama is found in Chapter 2 in the section Non-Woody Vegetation for Erosion and Sediment Control.

Figure TS-1 Geographical Areas for Species Adaptation and Seeding Dates

Note: Site conditions related to soils and aspect in counties adjacent to or close to county boundaries may justify adjustments in planting dates by qualified design professionals.

Table TS-l Commonly Used Plants for Temporary Cover

Species	Seeding Rate/AC PLS	North	Central	South
		Seeding Dates		
Millet, Browntop or German	40 lbs	Apr1-Aug 1	Apr1- Aug 15	Apr 1-Aug 15
Rye	3 bu	Sep I-Nov 15	Sep 15-Nov 15	Sep 15-Nov 15
Ryegrass	30 lbs	Aug l-Sep 15	Sep l-Oct 15	Sep 1-Oct 15
Sorghum-Sudan Hybrids	40 lbs	May l-Aug 1	Apr 15-Aug 1	Apr l-Aug 15
Sudangrass	40 lbs	May l-Aug I	Apr 15-Aug	Apr l-Aug 15
Wheat	3 bu	Sep I-Nov 1	Sep 15-Nov 15	Sep 15-Nov 15
Common Bermudagrass	10 lbs	Apr 1-July 1	Mar 15-July 15	Mar 1-July 15
Crimson Clover	10lbs	Sept 1-Nov 1	Sept 1-Nov 1	Sept 1-Nov 1

PLS means pure live seed and is used to adjust seeding rates. For example, to plant 10 lbs PLS of a species with germination of 80% and purity of 90%, PLS= 0.8X 0.9 = 72%. 10 lbs PLS = 10/0.72 = 13.9 lbs of the species to be planted.

Site Preparation and Soil Amendments

Complete grading and shaping before applying soil amendments if needed to provide a surface on which equipment can safely and efficiently be used to apply soil amendments and accomplish seedbed preparation and seeding.

Lime

Apply lime according to soil test recommendations. If a soil test is not available, use 1 ton of agricultural limestone or equivalent per acre on coarse textured soils and 2 tons per acre on fine textured soils. Do not apply lime to alkaline soils or to areas which have been limed during the preceding 2 years. Other liming materials that may be selected should be provided in amounts that provide equal value to the criteria listed for agricultural lime or be used in combination with agricultural limestone or Selma chalk to provide equivalent values to agricultural limestone.

Fertilizer

Apply fertilizer according to soil test results. If a soil test is not available, apply 8-24-24 fertilizer.

When vegetation has emerged to a stand and is growing, 30 to 40 lbs/acre (approximately 0.8 lbs/1000 ft^2) of additional nitrogen fertilizer should be applied.

Note: Fertilizer can be blended to meet exact fertilizer recommendations. Take soil test recommendations to local fertilizer dealer for bulk fertilizer blends. This may be more economical than bagged fertilizer.

Application of Soil Amendments

Incorporate lime and fertilizer into the top 6” of soil during seedbed preparation.

Seedbed Preparation

Good seedbed preparation is essential to successful plant establishment. A good seedbed is well pulverized, loose, and smooth. If soils become compacted during grading, loosen them to a depth of 6” to 8” using a ripper or chisel plow.

If rainfall has caused the surface to become sealed or crusted, loosen it just prior to seeding by disking, raking, harrowing, or other suitable methods. When hydroseeding methods are used, the surface should be left with a more irregular surface of clods.

Planting Methods

Seeding

Evenly apply seed using a cyclone seeder (broadcast), drill seeder, cultipacker seeder, or hydroseeder. Broadcast seeding and hydroseeding are appropriate for steep slopes where equipment cannot operate safely. Small grains should be planted no more than 1” deep, and grasses and legumes no more than ½” deep. Seed that are broadcast must be covered by raking or chain dragging, and then lightly firmed with a roller or cultipacker.

Hydroseeding

Surface roughening is particularly important when hydroseeding, as a roughened slope will provide some natural coverage for lime, fertilizer, and seed. The surface should not be compacted or smooth. Fine seedbed preparation is not necessary for hydroseeding operations; large clods, stones, and irregularities provide cavities in which seeds can lodge.

Mix seed, inoculant if required, and a seed carrier with water and apply as slurry uniformly over the area to be treated. The seed carrier should be a cellulose fiber, natural wood fiber or other approved fiber mulch material which is dyed an appropriate color to facilitate uniform application of seed. Use the correct legume inoculant at 4 times the recommended rate when adding inoculant to hydroseeder slurry. The mixture should be applied within one hour after mixing to reduce damage to seed.

Fertilizer should not be mixed with the seed-inoculant mixture because fertilizer salts may damage seed and reduce germination and seedling vigor. Fertilizer may be applied with a hydro seeder as a separate operation after seedlings are established.

Mulching

The use of appropriate mulch provides instant cover and helps ensure establishment of vegetative cover under normal conditions and is essential to seeding success

under harsh site conditions (see the Mulching practice for guidance). Harsh site conditions include the following: slopes steeper than 3:1 and adverse soils (soils that are shallow to rock, rocky, or high in clay or sand). Areas with concentrated flow should be treated differently and require a practice appropriate for channel flow. (refer to Chapter 5 Runoff Conveyance for guidance).

E. Preservation of Vegetation (PV)

Preservation of Vegetation (PV)

Practice Description

Preservation of vegetation is the avoidance of an area during land disturbing and construction activity to prevent mechanical and other injury to desirable plants in the planned landscape. The practice provides erosion and sediment control and is applicable where vegetative cover is desired and the existing plant community is compatible with the planned landscape.

Planning Considerations

Preservation of vegetation requires good site management to minimize the impact of construction activities on existing vegetation.

Plants to save should be identified prior to any construction activity.

Proper maintenance, especially during construction, is important to ensure healthy vegetation that can control erosion.

Different species, soil types, and climatic conditions will require different maintenance activities.

Design Criteria

Mark Plant Area for Retention

Groups of plants and individual trees to be retained should be located on a plan map. Limits of clearing should be planned outside the drip line of groups or individual trees to be saved. The clearing should never be closer than 5 feet to the trunk of a tree.

Flagging or other appropriate means of marking the site of the groups of plants and individual trees to be retained should be required before construction begins Individual trees to be retained should be marked with a highly visible paint or surveyor's ribbon in a band circling the tree at a height visible to equipment operators.

Plant Protection

Restrict construction equipment, vehicular traffic, stockpiles of construction materials, topsoil etc., from the areas where plants are retained and restrict these activities from occurring within the drip line of any tree to be retained. Trees being removed shall not be pushed into trees to be retained. Equipment operators shall not clean any of their equipment by slamming it against trees to be retained.

Restrict burning of debris within 100 feet of the plants being preserved. Fires shall be limited in size to prevent damage to any nearby trees.

Toxic material shall not be stored any closer than 100 feet to the drip line of any trees to be retained. Toxic materials shall be managed and disposed of according to state laws.

Fencing and Armoring

Groups of plants and trees should be protected by fencing or armoring where necessary (See Figure PV-l). The following types of fencing or armoring may be used:

- Board Fence-Board fence may be constructed with 4" square posts set securely in the ground and protruding at least 4 feet above the ground. A minimum of 2 horizontal boards should be placed between the posts. The fence should be placed at the limits of the clearing around the drip line of the tree. If it is not practical to erect a fence at the drip line, construct a triangular fence near the trunk. The limits of clearing will still be the drip line as the root zone within the drip line will still require protection.

- Cord Fence-Posts at least 2" square or 2" in diameter set securely in the ground and protruding at least 4 feet above the ground shall be placed at the limits of clearing with 2 rows of cord ¼" or thicker at least 2 feet apart running between posts with strips of surveyor's tape tied securely to the string at intervals of 3 feet or less.

- Earth Berms-Temporary earth berms may be constructed. The base of the berm on the tree side should be located along the limits of clearing. Earth berms may not be used for this purpose if their presence will create drainage patterns that cause erosion.

- Additional Trees-Additional trees may be left standing as protection between the trees to be retained and the limits of clearing. However, for this alternative to be used, trees in the buffer must be no more than 6 feet apart to prevent passage of equipment and material through the buffer.

- Plan for these additional trees to be evaluated prior to the completion of construction and either given sufficient treatment to ensure survival or be removed.

- Trunk Armoring-As a last resort, a tree may be armored with burlap wrapping and 2” studs wired vertically no more than 2” apart to a height of 5 feet. The armoring should encircle the tree trunk. Nothing should ever be nailed to a tree. The root zone within the drip line will still require protection.

- Fencing and armoring devices should be in place before any construction work is done and should be kept in good condition for the duration of construction activities. Fencing and armoring should not be removed until the completion of the construction project.

Raising the Grade

When the ground level must be raised around an existing tree or group of trees several methods may be used to insure survival.

A well may be created around a group of trees or an individual tree slightly beyond the drip line to retain the natural soil around the feeder roots (see Figure PV-2). When the well alternative is not practical or desirable, remove vegetation and organic matter from beneath the tree or trees for 3 feet beyond the drip line and loosen the surface soil to a depth of approximately 3” without damaging the roots.

Apply fertilizer in the root area of the tree to be retained. A soil test is the best way to determine what type of fertilizer to use. In the absence of a soil test, fertilizer should be applied at the rate of 1 to 2 pounds of 10-8-6 or 10-6-4 per inch of diameter at breast height (dbh) for trees under 6” dbh and at the rate of 2 to 4 pounds of 10-8-6 or 10-6-4 per inch of dbh for trees over 6” dbh.

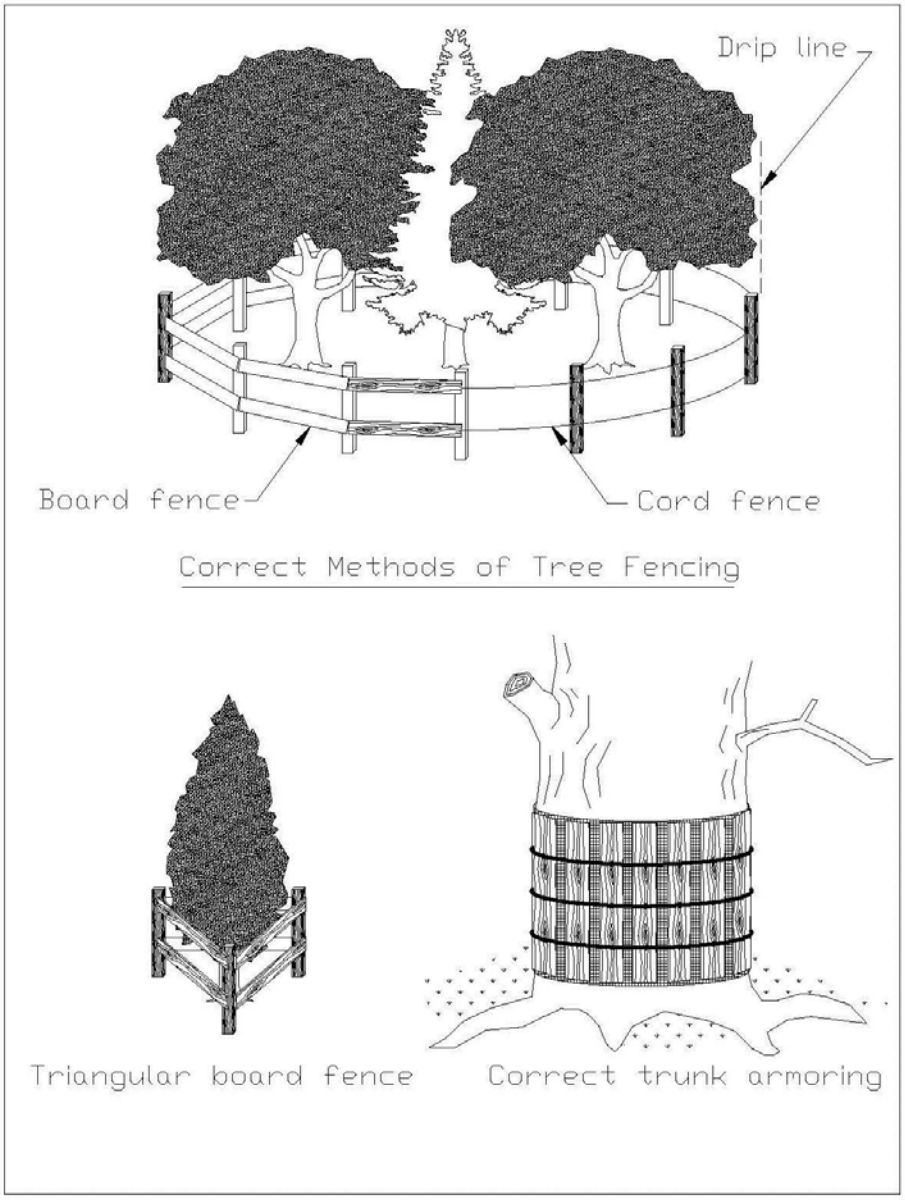


Figure PV-1 Fencing and Armoring

A dry well shall be constructed to allow for tree trunk diameter growth (see Figure PV-3). A space of at least 1 foot between the tree trunk and the well wall is adequate for old, slow growing trees. Clearance for younger trees shall be at least 2 feet. The well shall be high enough to bring the top just above the level of the proposed fill. The well wall shall taper slightly away from the tree trunk at a rate of 1” per foot of wall height.

The well wall shall be constructed of large stones, brick, building tile, concrete blocks, or cinder blocks. Openings should be left through the wall of the well to allow for free movement of air and water. Mortar shall only be used near the top of the well and only above the porous fill.

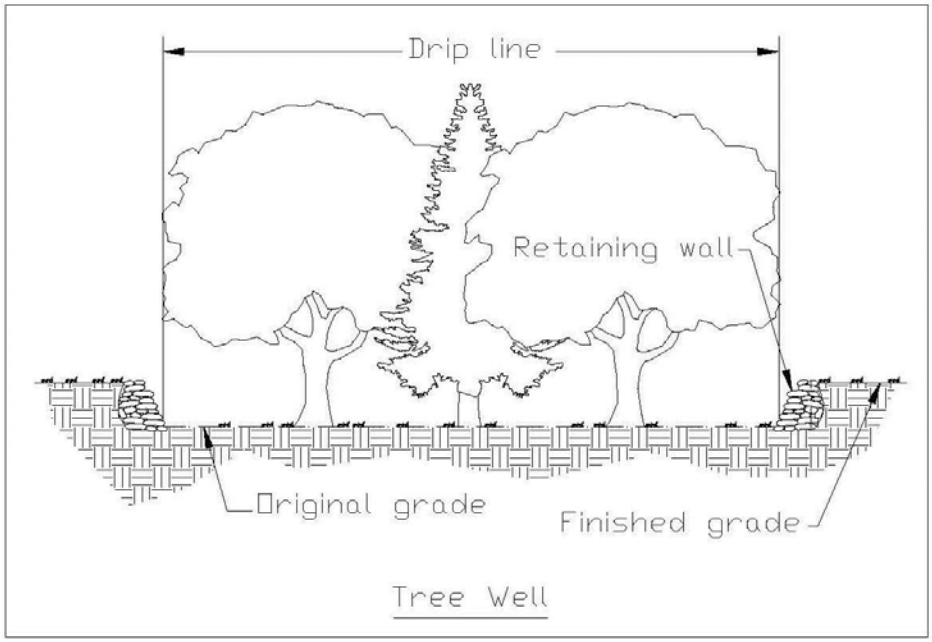


Figure PV-2 Tree Well

Drain lines composed of 4" high quality drain tiles shall begin at the lowest point inside the well and extend outward from the tree trunk in a wheel and spoke pattern with the trunk as the hub. Radial drain lines shall slope away from the well at a rate of ⅛" per foot. The circumference line of tiles should be located beneath the drip line of the trees. Vertical tiles or pipes shall be placed over the intersections of the two tile systems if a fill of more than 2 feet is contemplated. Vertical tiles shall be held in place with stone fill. Tile joints shall be tight. A few radial tiles shall extend beyond each intersection and shall slope sharply downward to insure good drainage. Tar paper or its approved equivalent shall be placed over the tile and/or pipe joints to prevent clogging and large stone shall be placed around and over drain tiles and/or pipes for protection.

A layer of 2" to 6" of stone shall be placed over the entire area under the tree from the well outward at least as far as the drip line. For fills up to 2 feet deep, a layer of stone 8" to 12" thick should be adequate.

A thick layer of this stone not to exceed 30" will be needed for deeper fills. A layer of ¾" to 1" stone covered by straw, fiberglass mat or a manufactured filter fabric shall be used to prevent soil from clogging the space between stones. Cinders shall not be used as fill material. Filling shall be completed with porous soil such as topsoil until the desired grade is reached. This soil shall be suitable to sustain specified vegetation.

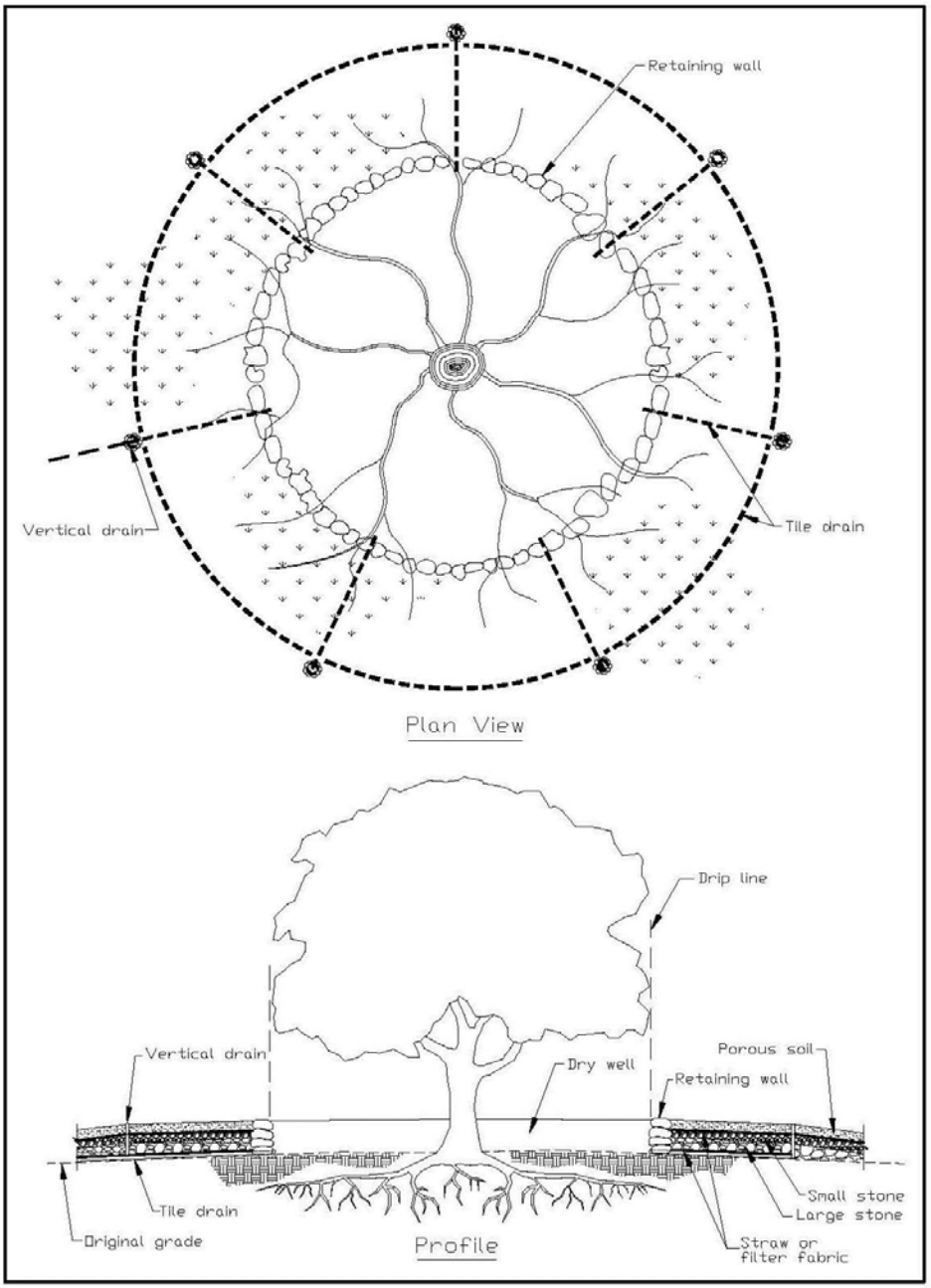


Figure PV-3 Tree Well Detail

Crushed stone shall be placed inside the dry well over the openings of the radial tiles to prevent clogging. The area between the trunk and the well wall shall either be covered by an iron grate or filled with a 50-50 mixture of crushed charcoal and sand to prevent anyone from falling into the dry well.

Where water drainage through the soil is not a problem, coarse gravel in the fill may be substituted for the tile. This material has sufficient porosity to ensure air drainage. Instead of the vertical tiles or pipes in the system, stones, crushed rock and gravel may be added so that the upper level of these porous materials slants toward the surface in the vicinity below the drip line.

Raising the grade on only one side of a tree or group of trees may be accomplished by constructing only half of one of these systems.

Lowering the Grade

Shrubs and trees shall be protected from the harmful grade cuts by the construction of a tree wall (see Figure PV-4). Following excavation, all tree roots that are exposed and/or damaged shall be trimmed cleanly and covered with moist peat moss, burlap or other suitable material to keep them from drying out.

The wall shall be constructed of large stones, brick, building tile, concrete block or cinder block. The wall should be backfilled with topsoil, peat moss, or other organic matter to retain moisture and aid in root development. Apply fertilizer and water thoroughly. The tree plants should be pruned to reduce the leaf surface in proportion to the amount of root loss. Drainage should be provided through the wall so water will not accumulate behind the wall. Lowering the grade on one side of the tree or group of trees can be accomplished by constructing only half of this system.

Trenching and Tunneling

Trenching should be done as far away from the trunks of trees as possible, preferably outside the branches or crown spreads of trees, to reduce the amount of root area damaged or killed by trenching activities. When possible, trenches should avoid large roots or root concentrations. This can be accomplished by curving the trench or by tunneling under large roots and areas of heavy root concentration. Tunneling under a species that does not have a large tap root may be preferable to trenching beside it as it has less impact on root systems (see Figure PV-5).

Roots should not be left exposed to the air but should be covered with soil as soon as possible or protected and kept moist with burlap or peat moss until the trench or tunnel can be filled. The ends of damaged and cut roots shall be cut off smoothly and moist peat moss, burlap or topsoil should be placed over the exposed area.

Trenches and tunnels shall be filled as soon as possible. Care should be taken to ensure that air spaces are not left in the soil. Peat moss or other organic matter shall be added to the fill material as an aid to inducing and developing root growth. The tree should be fertilized and mulched to stimulate new root growth and enhance general tree vigor. If a large part of the root system has been damaged the crown leaf surface area should be reduced in proportion to the root damage. This may be accomplished by pruning 20-30 percent of the crown foliage. If the roots are damaged during the winter the crown should be pruned before the next growing season. If roots are cut during the growing season, pruning should be done immediately.

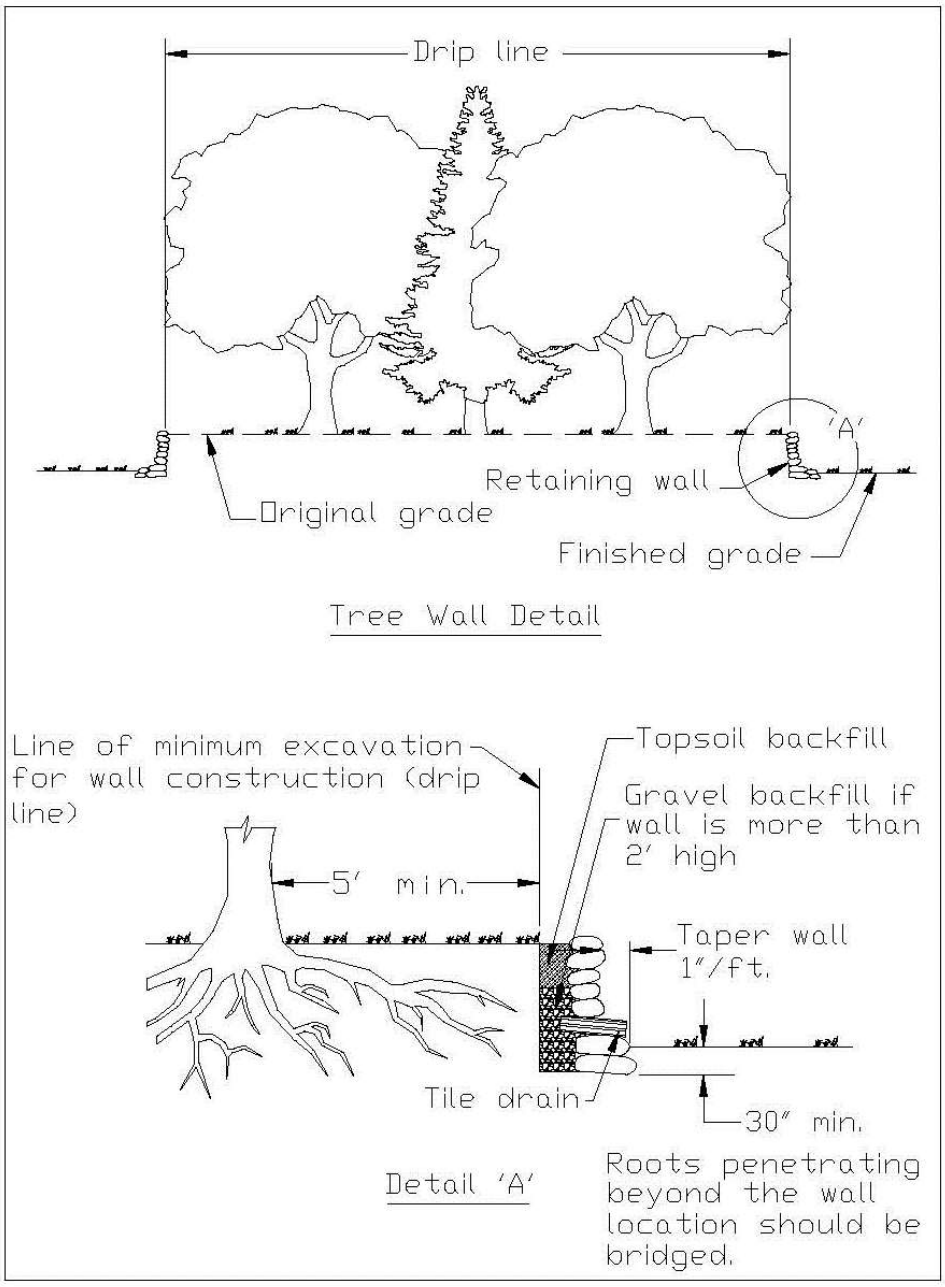


Figure PV-4 Tree Wall Detail

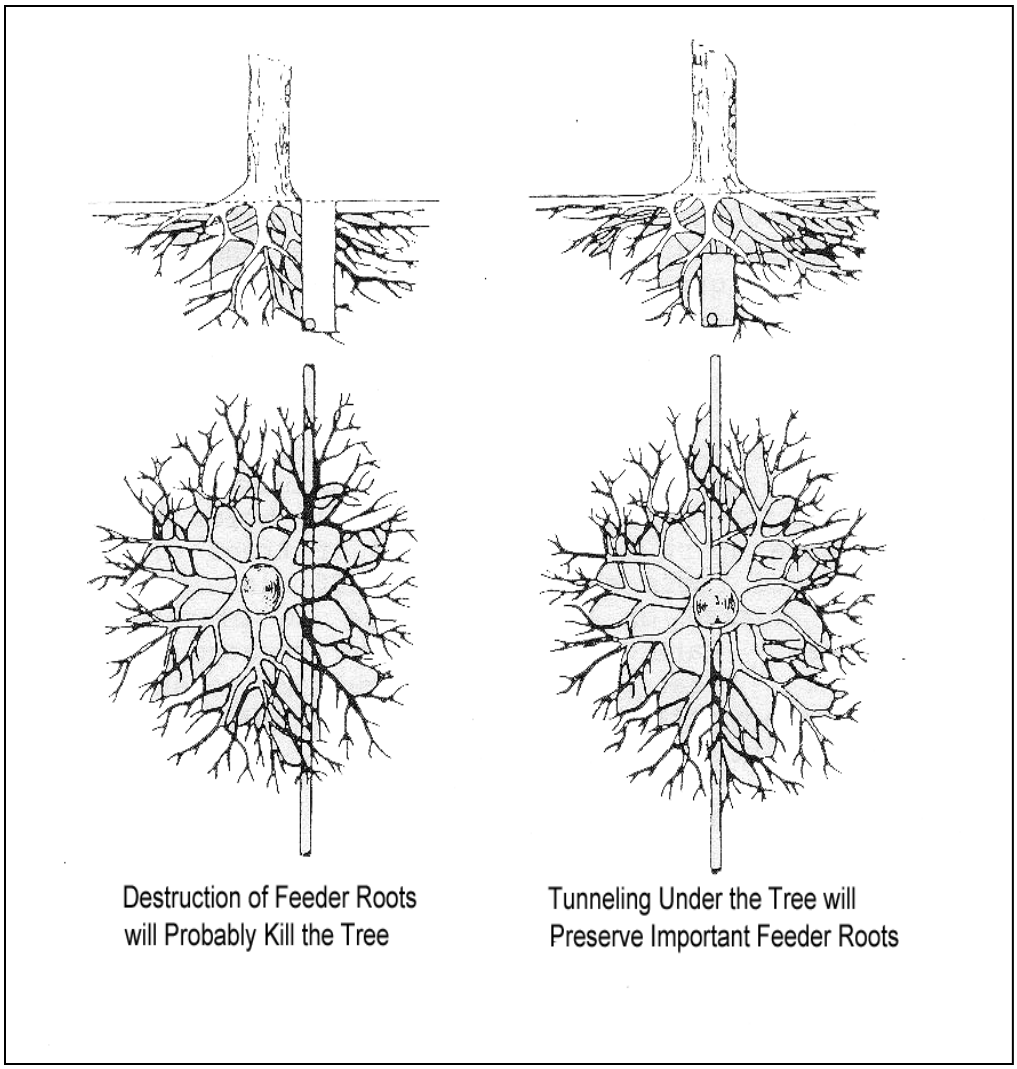


Figure PV-5 Trenching vs Tunneling

Treating Damaged Trees

When trees are damaged during construction activities certain maintenance practices can be applied to protect the health of the tree.

Soil aeration may be needed if the soil has been compacted. The soil around trees can be aerated by punching holes 1 foot deep and 18" apart under the crown of trees with an iron pipe.

Damaged roots should be cut off cleanly and moist peat moss, burlap or topsoil should be placed over the exposed area. Bark damage should be treated by removing loose bark.

Tree limbs damaged during construction or removed for any other reason shall be cut off above the collar at the branch junction.

Trees that have been stressed or damaged should be fertilized to aid their recovery.

Trees should be fertilized in the spring or fall. Fall applications are preferred.

Fertilizer should be applied to the soil over the feeder roots. In no case should it be applied closer than 3 feet to the trunk. Root systems of trees extend some distance beyond the drip line. The area to be fertilized should be increased by ¼ the area of the crown. A soil test is the best way to determine what type of fertilizer to use. In the absence of a soil test, fertilizer should be applied at the rate of 1 to 2 pounds of 10-8-6 or 10-6-4 per inch of dbh for trees under 6” dbh and at the rate of 2 to 4 pounds of 10-8-6 or 10-6-4 per inch of dbh for trees over 6” dbh.

A ground cover or organic mulch layer should be maintained around trees to prevent erosion, protect roots and to conserve water.

F. Check Dam (CD)

Check Dam (CD)

Practice Description

A check dam (also referred to as a “ditch check”) is a small barrier or dam constructed across a swale, drainage ditch or other area of concentrated flow for the purpose of reducing channel erosion. Channel erosion is reduced because check dams flatten the gradient of the flow channel and slow the velocity of channel flow. Check dams do not reduce turbidity of runoff. Check dams can be constructed of rock, wattles (sometimes referred to as tubes or rolls), sand bags, or other materials that may be acceptable to the design professional. Unless installed correctly, check dams will not capture a significant amount of sediment. When installed correctly, most check dams can capture the coarser grained material, which can be significant for sandy soils. Sediment capture increases as velocity in the channel decreases by creating impoundments with the check dams. This impoundment pool creates the flattening of the gradient, greatly reducing channel erosion.

This practice applies in small open channels and drainageways, including temporary and permanent swales. Check dams are not to be used in a live stream. Situations of use include areas in need of protection during establishment of grass and areas that cannot receive a temporary or permanent non-erodible lining for an extended period.

Planning Considerations

Check dams are used in concentrated flow areas to provide temporary channel stabilization with minimal sediment retention during rainfall runoff periods on construction sites. Check dams may be constructed of rock, wattles, sand bags, or other suitable material, including manufactured products. Water flowing over a check dam creates turbulent erosive forces (super critical flow) that must be addressed to prevent erosion downstream of the check dam. Inevitably water will likely flow under check dams due to limitation with ground contact. Therefore, it is of upmost importance to ensure the performance of the check dam that erosion and scour under the check dam be minimized. This is best achieved using an underlay such as an 8-oz. nonwoven filter fabric. If the underlay is extended downstream, it will also protect the channel from super critical flows from water flowing over and under the dam.

Check dams should be planned to be compatible with the other features such as streets, walkways, trails, sediment basins and rights-of-way or property lines. Check dams are installed with the center overflow area lower in elevation than the ends to ensure flow goes over the check dam and not around. Check dams are normally constructed in series and the dams should be located at a normal interval from other grade controls such as culverts or sediment basins.

Check dams are generally used as a temporary BMP that is removed following construction to allow for final long-term stabilization of the channel. Provisions should be made to establish permanent channel linings as early as possible.

Check dams can also be used for other purposes such as the capture of sediment upstream of other practices or flocculent dosing upstream of a sediment basin.

Extensive research has been conducted by The Auburn University Erosion and Sediment Control Test Facility. The research recommendations are incorporated in the following planning considerations:

Rock Check Dams

Many check dams are constructed of rock. Rock may not be acceptable in some installations and alternative types of check dams need to be considered. Rock check dams (Figures CD-1 and CD-2) are usually installed with mechanical equipment but hand labor is likely needed to complete most installations to the quality needed. The availability and cost of commercially produced rock should be considered. The use of rock should be considered carefully in areas to be mowed. Some rock may be washed downstream and should be removed before each mowing operation. The use of geotextile can be used on the upstream face of the rock check dam to increase the sediment trapping efficiency of the rock check dam. Measures must be taken to prevent undermining of the check dam and erosion below the check dam. A non-woven geotextile underlayment should be used to prevent this from happening. The geotextile meeting AASHTO M 288 requirement for separation Class II (minimum 8-oz. fabric) should extend approximately 3 ft. upstream and downstream, and pinned securely with the upstream edge buried.

Measures to prevent downstream erosion associated with a rock check dam include placing larger rock on the downstream face of a rock dam, and providing erosion protection material just downstream of the dam.

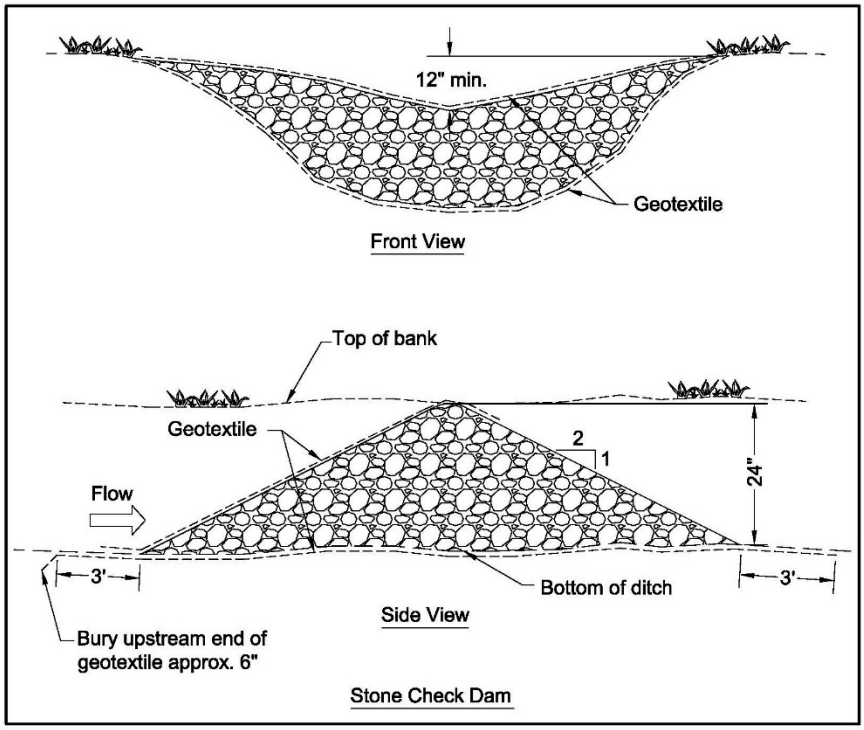


Figure CD-1 Profile and Cross-Section of Typical Rock Check Dams

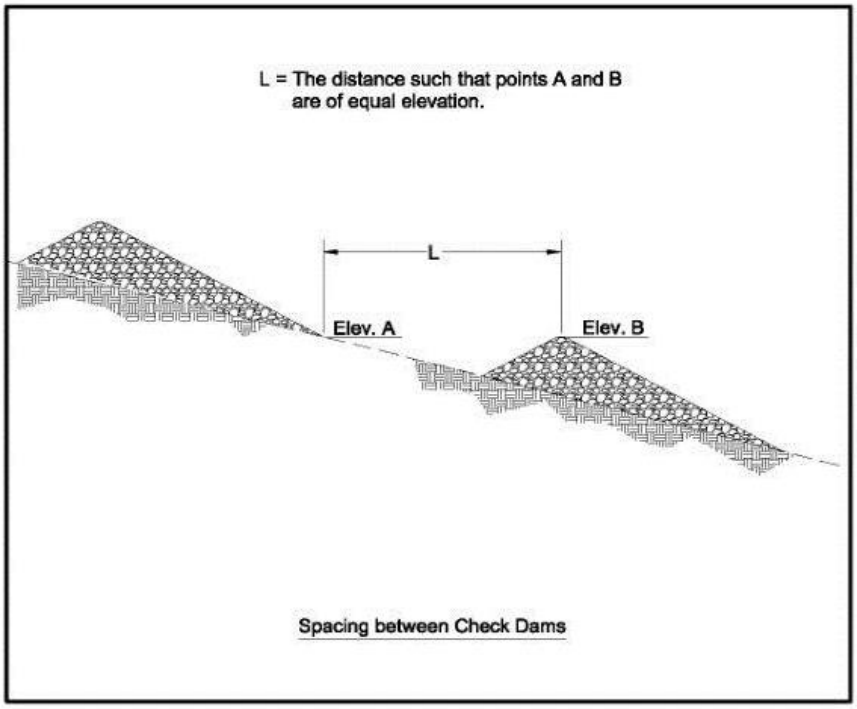


Figure CD-2 Profile of Typical Rock Check Dams

Wattle Check Dams

Wattles have been found to be best installed without trenching and on top of stapled geotextile underlayment that extends a minimum 3 ft. up and downstream from the wattle. Wattles must be properly stapled with sod staples on 10-inch centers on each side of the wattle to prevent flotation, and staked over the top using non-destructive tee-pee type staking. Wattles that provide less “flow through” create more ponding of water that increases the trapping of sediment (see Figures CD-3 and CD-4).

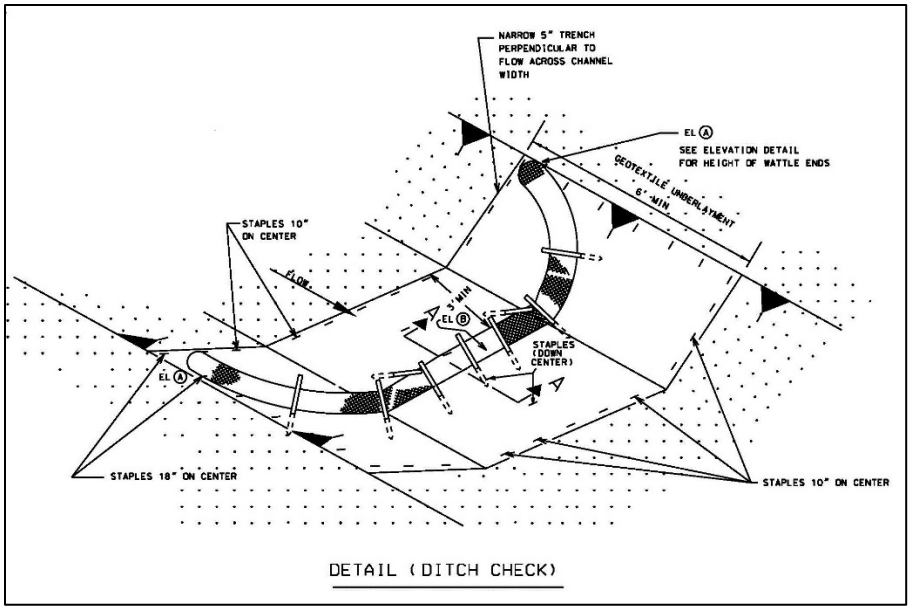


Figure CD-3 Wattle Check Dam (ditch check)

Figure CD-4 Wattle Check Dam (ditch check)
(Photo courtesy of Auburn University Erosion and Sediment Control Test Facility)

Silt Fence Check Dam

When properly designed and installed, typical silt fence materials can be utilized to construct a check dam. Geotextile underlayment should be used and the fence notched as needed to ensure the maximum depth of flow is no greater than the depth of the channel. Figures CD-5 and CD-6 show the recommended details.

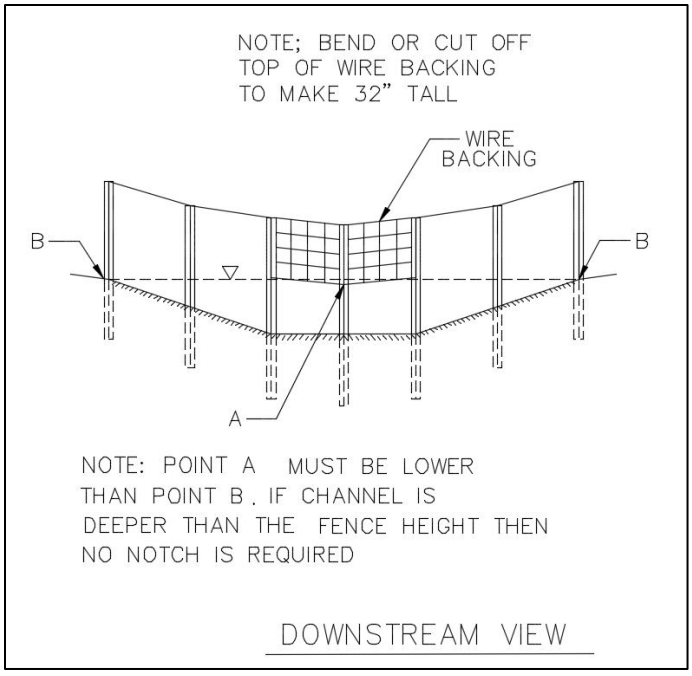


Figure CD-5 Silt Fence Check Dam Cross-Section

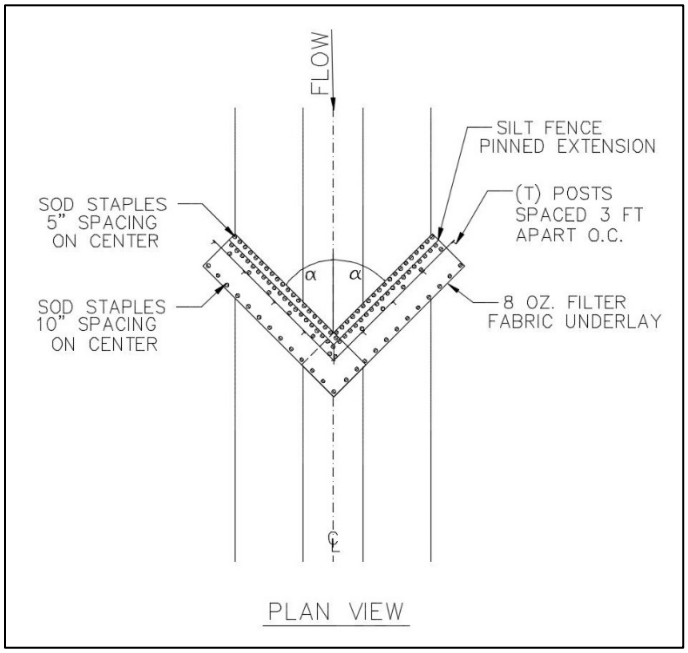


Figure CD-6 Silt Fence Check Dam Plan View

Figure CD-7 Silt Fence Check Dam
(Photo courtesy of Auburn University Erosion and Sediment Control Test Facility)

Sand Bag Check Dam

Sand bags have also been proven to be effective as check dams but only when the bags are properly oriented (See Figures CD-8 and CD-9). A geotextile underlayment that extends approximately 3 ft. upstream and downstream should also be used in earth channel situations to prevent undermining and scour.

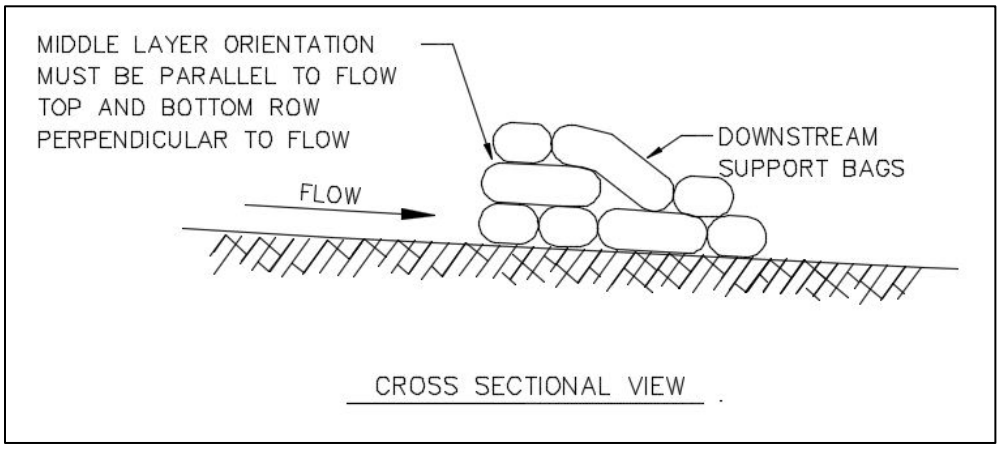


Figure CD-8 Sand Bag Check Dam Cross-Section

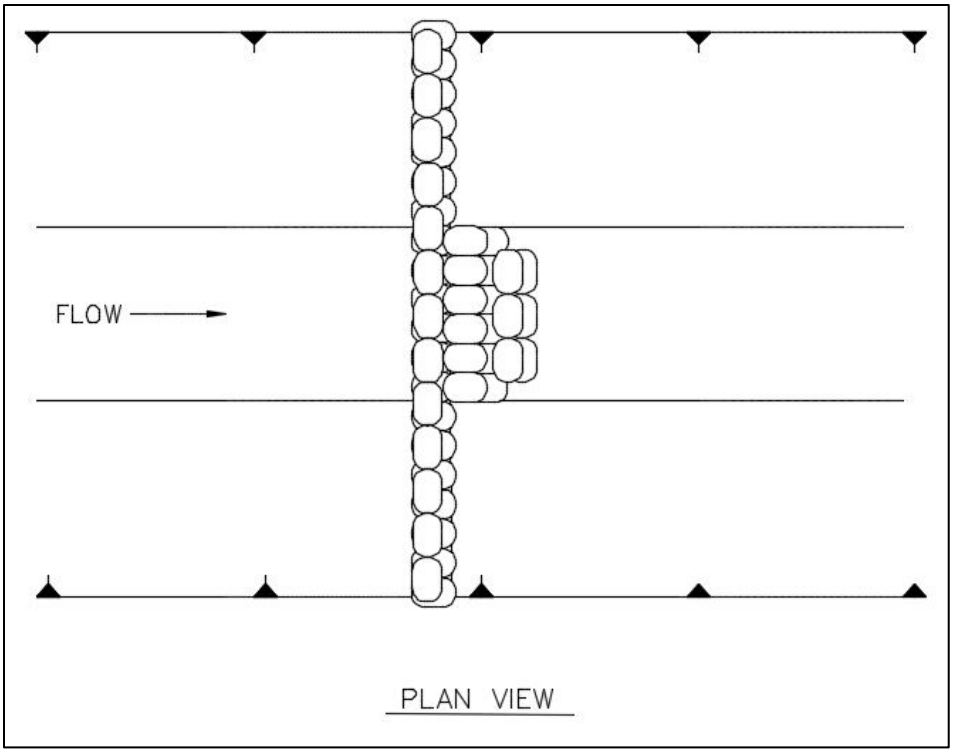


Figure CD-9 Sand Bag Check Dam Plan View

Design Criteria

Formal design is not required. The following factors should be considered when designing check dams.

Drainage Area

Generally, one acre or less.

Maximum Height

Check dam height is a function of channel geometry. Most check dams are 3 feet or less in height.

Depth of Flow

Depth of flow over a check dam is a function of the cross-section and porosity of the check dam. Generally, flows over a check dam are less than 1 foot.

The center of the dam should be constructed lower than the ends. The elevation of the center of the dam should be lower than the ends by the depth of design flow.

Side Slopes

2:1 or flatter (rock check dam).

Spacing

The elevation of the toe of the upstream dam should be at or below the elevation of crest of the downstream dam (Figure CD-2).

For example, if the channel is 3% grade, and the check dam height is 2 feet, The check dam spacing should be 67 feet:

Spacing (ft) = dam height (ft) / channel grade

Spacing = 2 ft / 0.03 = 67 feet

Geotextile

Generally, the non-woven geotextile should meet the requirements found in AASHTO M 288 Class II used for separation.

G. Diversion (DV)

Diversion (DV)

Practice Description

A diversion is a watercourse constructed across a slope consisting of an excavated channel, a compacted ridge or a combination of both. Most diversions are constructed by excavating a channel and using the excavated material to construct a ridge on the downslope side of the channel. Right-of-way diversions and temporary diversions are sometimes constructed by making a ridge, often called a berm, from fill material.

This practice applies to sites where stormwater runoff can be redirected to permanently protect structures or areas downslope from erosion, sediment, and excessive wetness or localized flooding. Diversions may be used to temporarily divert stormwater runoff to protect disturbed areas and slopes or to retain sediment on-site during construction.

Perimeter protection is sometimes used to describe both permanent and temporary diversions used at either the upslope or downslope side of a construction area.

Right-of-way diversions, sometimes referred to as water bars, are used to shorten the flow length on a sloping right-of-way and reduce the erosion potential of the stormwater runoff.

Planning Considerations

Diversions are designed to intercept and carry excess water to a stable outlet.

Diversions can be useful tools for managing surface water flows and preventing soil erosion. On moderately sloping areas, they may be placed at intervals to trap and divert sheet flow before it has a chance to concentrate and cause rill and gully erosion.

Diversions may be placed at the top of cut or fill slopes to keep runoff from upgradient drainage areas off the slope. The following picture illustrates the placement of a diversion near the top of the slope. Diversions are sometimes built at the base of steeper slopes to protect flatter developed areas which cannot withstand runoff water from outside areas. Also, they can be used to protect structures, parking lots, adjacent properties, and other special areas from flooding.

Figure DV-1 Diversion near the top of a slope

Diversions are preferable to other types of man-made stormwater conveyance systems because they more closely simulate natural flow patterns and characteristics. Flow velocities are generally kept to a minimum. When properly coordinated into the landscape design of a site, diversions can he visually pleasing as well as functional.

As with any earthen structure, it is very important to establish adequate vegetation as soon as possible after installation. It is usually important to stabilize the drainage area above the diversion so that sediment will not enter and accumulate in the diversion channel.

Design Criteria

Location

Diversion location should be determined by considering outlet conditions, topography, land use, soil type, length of slope, seepage (where seepage is a problem) and the development layout. Outlets must be stable after the diversion empties stormwater flow into it; therefore, care should be exercised in selecting the location of the diversion and its outlet.

Capacity

The diversion channel must have a minimum capacity to carry the runoff expected from a storm frequency meeting the requirements of Table DV-l with a freeboard of at least 0.3 foot (Figure DV-1).

The storm frequency should be used to determine the required channel capacity, Q (peak rate of runoff). The peak rate of runoff should be determined using the Natural Resources Conservation Service runoff curve no. (RCN) method or other equivalent methods.

Table DV-1 Design Frequency

Diversion Type	Typical Area of Protection	24-Hour Design Storm Frequency
Temporary	Construction Areas	2-year
	Building Sites	5-year
Permanent	Agricultural Land	10-year
	Mined Reclamation Area	10-year
	Recreation Areas	10-year
	Isolated Buildings	25-year
	Urban areas, Residential, School, Industrial Areas, etc.	50-year

Diversions designed to protect homes, schools, industrial buildings, roads, parking lots, and comparable high-risk areas, and those designed to function in connection with other structures, should have sufficient capacity to carry peak runoff expected from a storm frequency consistent with the hazard involved.

Velocities

Diversions should be designed so that the design velocities will be safe for the planned type of protective vegetation and the expected maintenance. Maximum permissible velocities are dependent upon the erosion resistance of the soil (Table DV-2) and the quality of the vegetation maintained.

Table DV-2 Permissible Velocities

Soil Texture	Velocity in Feet/Second		
	Conditions of Vegetation		
	Poor	Fair	Good
Sand, Silt, Sandy Loam, Silt Loam	1.5	2.0	3.0
Silty Clay Loam, Sandy Clay Loam	2.5	3.0	4.0
Clay	3.0	4.0	5.0

Channel Design

The diversion channel may be parabolic, trapezoidal or v-shaped as shown in Figure DV-2 and should be designed in accordance with the procedure provided in the Diversion Design section. Land slope must be considered when choosing channel dimensions. On steeper slopes, narrow and deep channels may be required. On more gentle slopes, broad, shallow channels can be used to facilitate maintenance.

Ridge Design

The supporting ridge cross section should meet the configuration and requirements of Figure DV-2.

The side slopes should be no steeper than 2:1. Side slopes should be flatter, 5:1 to 10:1, when the diversion is to be permanent with mowing and other maintenance activities performed on or around it.

The width of the ridge at the design water elevation should be a minimum of 4 feet.

The minimum freeboard should be 0.3 foot.

The design should include a 10% settlement factor.

Outlet

Diversions should have adequate outlets which will convey concentrated runoff without erosion. Acceptable outlets include practices such as Grassed Swale, Lined Swale, Drop Structure, Sediment Basin, and Stormwater Detention Basins.

Stabilization

Unless otherwise stabilized, the ridge and channel should be seeded within 13 days of installation in accordance with the applicable seeding practice, Permanent Seeding or Temporary Seeding.

Disturbed areas draining into the diversion should be seeded and mulched prior to or at the time the diversion is constructed in accordance with the Permanent Seeding or Temporary Seeding (whichever is applicable) practices.

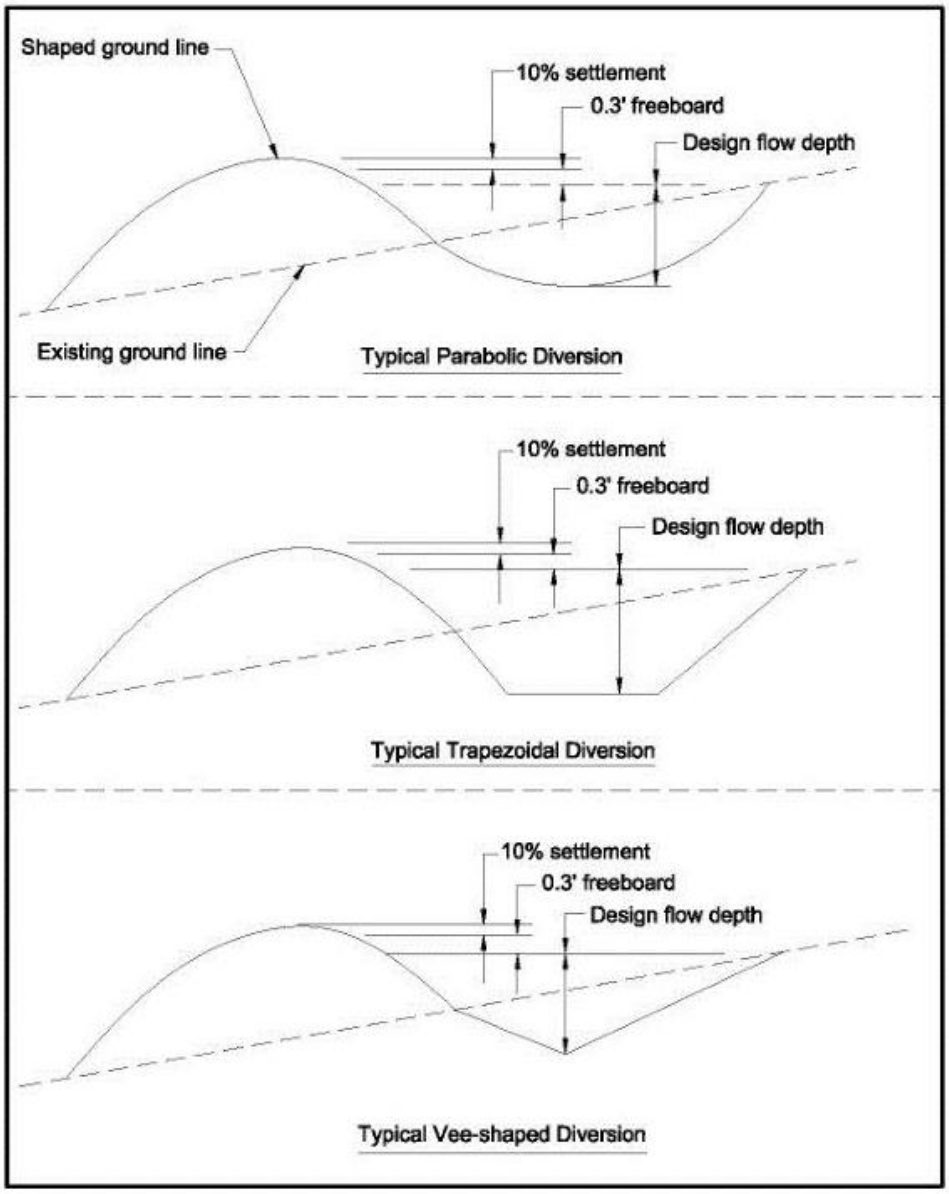


Figure DV-2 Typical Diversions Detail

Diversion Design

Note: This design example uses the Permissible Velocity approach. Diversion design using the Tractive Stress approach can also be used but is not discussed in this document.

Table DV-l through DV-16 may be used to facilitate the design of grass-lined diversions with parabolic cross sections. These tables are based on a retardance of "D" (vegetation newly cut) to determine V1 for stability considerations. To determine channel capacity, choose a retardance of "C" when proper maintenance is expected; otherwise, design channel capacity based on retardance "B". Refer to Table DV-2 for maximum permissible velocities. The permissible velocities guide the selection of V1 and should not be exceeded. It is good practice to use a value for V1 that is significantly less than the maximum allowable when choosing a design cross section. When velocities approach the maximum allowable, flatter grades should be evaluated or a more erosion resistant liner such as erosion control blanket or riprap should be considered. After the diversion dimensions are selected in the design tables, the top width should be increased by 4 feet. and the depth by 0.3 foot. for freeboard.

Example Problem

Given

Q: 30 cfs
Grade: 1%
Soil: Sandy clay loam
Condition of vegetation expected: fair
Maintenance: low; will be cut only twice a year.
Site will allow a top width of 26 feet.

Find

Diversion top width and depth that will be stable and fit site conditions.

Solution

From Table DV-2 use maximum permissible velocity of 3.0 ft./sec.

Since maintenance will be low use "B" retardance for capacity.

From Table DV-4 use retardance "D" and "B";
Grade 1.00 Percent. Top width = 21.0 feet + 4 feet = 25.0 feet.

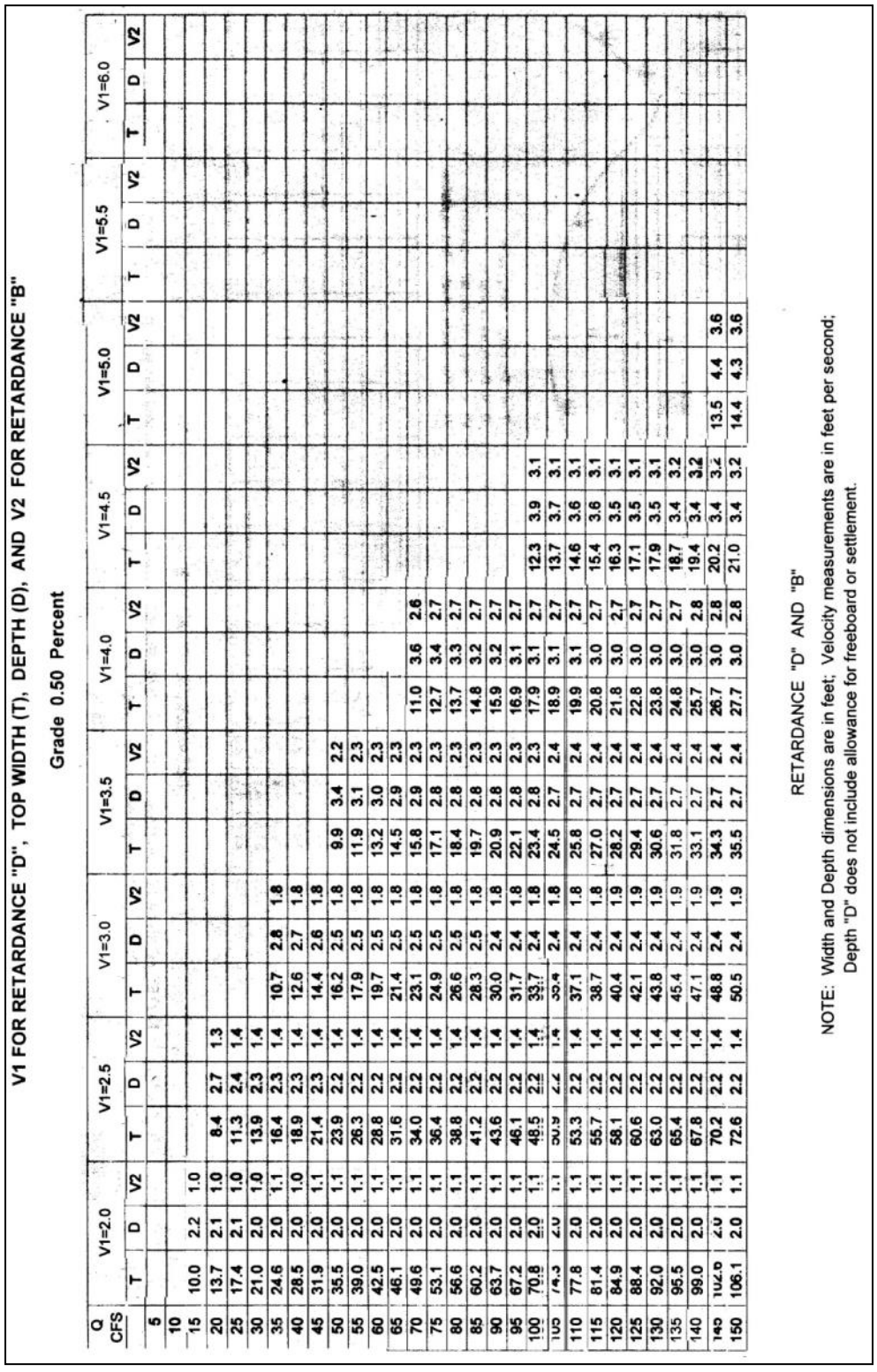
Depth = 1.6 feet + 0.3 foot = 1.9 feet.

V_2 = 1.3 ft./sec.

Note: V_1 < 3.0 ft./sec.; Top width < 26 feet, design O.K.

Note: It is good practice to select a cross section that will give a velocity, V_1, well below the maximum allowable whenever site conditions permit. Wide, shallow cross sections are more stable and require less maintenance. It is always prudent to evaluate flatter design grades to best fit diversions to the site and keep velocities well below maximum allowable.

Table DV-3 Parabolic Diversion Design Chart (Retardance “D” and “B”, Grade 0.50%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "B"

Grade 0.50 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5																											
10																											
15	10.0	2.2	1.0																								
20	13.7	2.1	1.0	8.4	2.7	1.3																					
25	17.4	2.1	1.0	11.3	2.4	1.4																					
30	21.0	2.0	1.0	13.9	2.3	1.4																					
35	24.6	2.0	1.1	16.4	2.3	1.4	10.7	2.6	1.8																		
40	28.5	2.0	1.0	18.9	2.3	1.4	12.6	2.7	1.8																		
45	31.9	2.0	1.1	21.4	2.3	1.4	14.4	2.6	1.8																		
50	35.5	2.0	1.1	23.9	2.2	1.4	16.2	2.5	1.8	9.9	3.4	2.2															
55	39.0	2.0	1.1	26.3	2.2	1.4	17.9	2.5	1.8	11.9	3.1	2.3															
60	42.5	2.0	1.1	28.8	2.2	1.4	19.7	2.5	1.8	13.2	3.0	2.3															
65	46.1	2.0	1.1	31.6	2.2	1.4	21.4	2.5	1.8	14.5	2.9	2.3															
70	49.6	2.0	1.1	34.0	2.2	1.4	23.1	2.5	1.8	15.8	2.9	2.3	11.0	3.6	2.6												
75	53.1	2.0	1.1	36.4	2.2	1.4	24.9	2.5	1.8	17.1	2.8	2.3	12.7	3.4	2.7												
80	56.6	2.0	1.1	38.8	2.2	1.4	26.6	2.5	1.8	18.4	2.8	2.3	13.7	3.3	2.7												
85	60.2	2.0	1.1	41.2	2.2	1.4	28.3	2.5	1.8	19.7	2.8	2.3	14.8	3.2	2.7												
90	63.7	2.0	1.1	43.6	2.2	1.4	30.0	2.4	1.8	20.9	2.8	2.3	15.9	3.2	2.7												
95	67.2	2.0	1.1	46.1	2.2	1.4	31.7	2.4	1.8	22.1	2.8	2.3	16.9	3.1	2.7												
100	70.8	2.0	1.1	48.5	2.2	1.4	33.7	2.4	1.8	23.4	2.8	2.3	17.9	3.1	2.7	12.3	3.9	3.1									
105	74.3	2.0	1.1	50.9	2.2	1.4	35.4	2.4	1.8	24.5	2.7	2.4	18.9	3.1	2.7	13.7	3.7	3.1									
110	77.8	2.0	1.1	53.3	2.2	1.4	37.1	2.4	1.8	25.8	2.7	2.4	19.9	3.1	2.7	14.6	3.6	3.1									
115	81.4	2.0	1.1	55.7	2.2	1.4	38.7	2.4	1.8	27.0	2.7	2.4	20.8	3.0	2.7	15.4	3.6	3.1									
120	84.9	2.0	1.1	58.1	2.2	1.4	40.4	2.4	1.9	28.2	2.7	2.4	21.8	3.0	2.7	16.3	3.5	3.1									
125	88.4	2.0	1.1	60.6	2.2	1.4	42.1	2.4	1.9	29.4	2.7	2.4	22.8	3.0	2.7	17.1	3.5	3.1									
130	92.0	2.0	1.1	63.0	2.2	1.4	43.8	2.4	1.9	30.6	2.7	2.4	23.8	3.0	2.7	17.9	3.5	3.1									
135	95.5	2.0	1.1	65.4	2.2	1.4	45.4	2.4	1.9	31.8	2.7	2.4	24.8	3.0	2.7	18.7	3.4	3.2									
140	99.0	2.0	1.1	67.8	2.2	1.4	47.1	2.4	1.9	33.1	2.7	2.4	25.7	3.0	2.8	19.4	3.4	3.2									
145	102.6	2.0	1.1	70.2	2.2	1.4	48.8	2.4	1.9	34.3	2.7	2.4	26.7	3.0	2.8	20.2	3.4	3.2	13.5	4.4	3.6						
150	106.1	2.0	1.1	72.6	2.2	1.4	50.5	2.4	1.9	35.5	2.7	2.4	27.7	3.0	2.8	21.0	3.4	3.2	14.4	4.3	3.6						

RETARDANCE "D" AND "B"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-4 Parabolic Diversion Design Chart (Retardance “D” and “B”, Grade 1.00%)

V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "B"

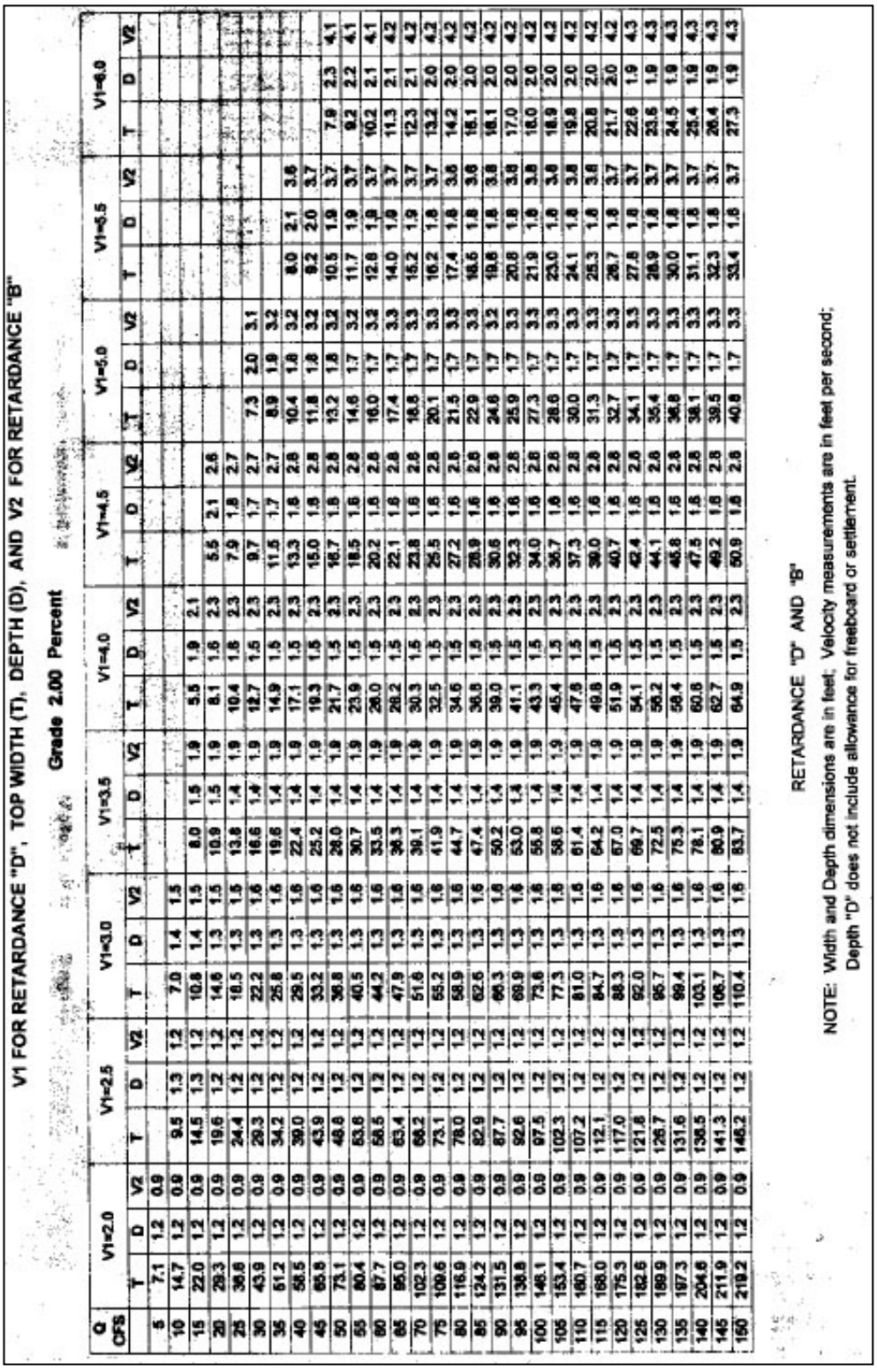
Grade 1.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5																											
10	9.7	1.6	1.0	6.2	2.0	1.2																					
15	14.8	1.5	1.0	10.2	1.7	1.3	6.5	2.2	1.5																		
20	20.2	1.5	1.0	13.8	1.7	1.3	9.6	1.9	1.6																		
25	25.1	1.5	1.0	17.4	1.7	1.3	12.2	1.9	1.6	8.5	2.2	2.0															
30	30.1	1.5	1.0	21.0	1.6	1.3	14.9	1.8	1.7	10.6	2.1	2.1															
35	35.1	1.5	1.0	24.7	1.6	1.3	17.5	1.8	1.7	12.6	2.0	2.1	8.9	2.4	2.5												
40	40.1	1.5	1.0	28.2	1.6	1.3	20.0	1.8	1.7	14.5	2.0	2.1	10.5	2.3	2.5												
45	45.1	1.5	1.0	31.7	1.6	1.3	22.5	1.8	1.7	16.4	2.0	2.1	12.1	2.2	2.5	8.2	2.8	2.9									
50	50.2	1.5	1.0	35.2	1.6	1.3	25.4	1.8	1.7	18.3	2.0	2.1	13.6	2.2	2.5	10.0	2.6	2.9									
55	55.2	1.5	1.0	38.8	1.6	1.3	27.9	1.8	1.7	20.3	1.9	2.1	15.1	2.2	2.5	11.2	2.5	3.0									
60	60.2	1.5	1.0	42.3	1.6	1.3	30.4	1.8	1.7	22.2	1.9	2.1	16.6	2.1	2.5	12.4	2.4	3.0									
65	65.2	1.5	1.0	45.8	1.6	1.3	32.9	1.8	1.7	24.0	1.9	2.1	18.0	2.1	2.5	13.6	2.4	3.0	8.9	3.1	3.5						
70	70.2	1.5	1.0	49.3	1.6	1.3	35.5	1.8	1.7	25.9	1.9	2.1	19.5	2.1	2.6	14.8	2.4	3.0	10.6	2.8	3.5						
75	75.2	1.5	1.0	52.8	1.6	1.3	38.0	1.8	1.7	28.2	1.9	2.1	20.9	2.1	2.6	16.0	2.3	3.0	11.5	2.8	3.5						
80	80.2	1.5	1.0	56.3	1.6	1.3	40.5	1.8	1.7	30.0	1.9	2.1	22.3	2.1	2.6	17.1	2.3	3.0	12.5	2.7	3.5						
85	85.2	1.5	1.0	59.8	1.6	1.3	43.0	1.8	1.7	31.9	1.9	2.1	23.7	2.1	2.6	18.3	2.3	3.0	13.5	2.7	3.6	9.8	3.3	3.9			
90	90.2	1.5	1.0	63.3	1.6	1.3	45.6	1.8	1.7	33.6	1.9	2.1	25.2	2.1	2.6	19.4	2.3	3.1	14.4	2.6	3.6	10.9	3.1	3.9			
95	95.2	1.5	1.0	66.9	1.6	1.3	48.1	1.8	1.7	35.5	1.9	2.1	26.6	2.1	2.6	20.5	2.3	3.1	15.3	2.6	3.6	12.0	3.0	3.9			
100	100.2	1.5	1.0	70.4	1.6	1.3	50.6	1.8	1.7	37.4	1.9	2.1	28.0	2.1	2.6	21.6	2.3	3.1	16.2	2.6	3.6	12.9	2.9	4.0			
105	105.3	1.5	1.0	73.9	1.6	1.3	53.1	1.8	1.7	39.2	1.9	2.1	29.8	2.1	2.6	22.8	2.3	3.1	17.1	2.6	3.6	13.7	2.9	4.0	10.8	3.4	4.3
110	110.3	1.5	1.0	77.4	1.6	1.3	55.7	1.8	1.7	41.1	1.9	2.1	31.3	2.1	2.6	23.9	2.3	3.1	18.0	2.6	3.6	14.4	2.9	4.0	12.0	3.2	4.3
115	115.3	1.5	1.0	80.9	1.6	1.3	58.2	1.8	1.7	42.9	1.9	2.1	32.7	2.1	2.6	25.0	2.3	3.1	18.9	2.5	3.6	15.2	2.8	4.0	12.7	3.2	4.3
120	120.3	1.5	1.0	84.4	1.6	1.3	60.7	1.8	1.7	44.8	1.9	2.1	34.1	2.1	2.6	26.1	2.2	3.1	19.7	2.5	3.6	16.0	2.8	4.0	13.4	3.1	4.3
125	125.3	1.5	1.0	88.0	1.6	1.3	63.2	1.8	1.7	46.7	1.9	2.1	35.5	2.1	2.6	27.2	2.2	3.1	20.6	2.5	3.6	16.8	2.8	4.0	14.1	3.1	4.3
130	130.3	1.5	1.0	91.5	1.6	1.3	65.8	1.8	1.7	48.5	1.9	2.1	36.9	2.1	2.6	28.4	2.2	3.1	21.5	2.5	3.6	17.4	2.8	4.0	14.8	3.1	4.3
135	135.3	1.5	1.0	95.0	1.6	1.3	68.3	1.8	1.7	50.4	1.9	2.1	38.3	2.1	2.6	29.5	2.2	3.1	22.4	2.5	3.6	18.2	2.8	4.0	15.5	3.0	4.3
140	140.3	1.5	1.0	98.5	1.6	1.3	70.8	1.8	1.7	52.2	1.9	2.1	39.7	2.0	2.6	30.6	2.2	3.1	23.2	2.5	3.6	18.9	2.7	4.0	16.1	3.0	4.4
145	145.3	1.5	1.0	102.0	1.6	1.3	73.3	1.8	1.7	54.1	1.9	2.1	41.1	2.0	2.6	32.1	2.2	3.0	24.1	2.5	3.6	19.7	2.7	4.0	16.8	3.0	4.4
150	150.3	1.5	1.0	105.5	1.6	1.3	75.9	1.8	1.7	56.0	1.9	2.1	42.5	2.0	2.6	33.2	2.2	3.0	25.0	2.5	3.6	20.4	2.7	4.1	17.5	2.9	4.4

RETARDANCE "D" AND "B"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-5 Parabolic Diversion Design Chart (Retardance “D” and “B”, Grade 2.00%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "B"

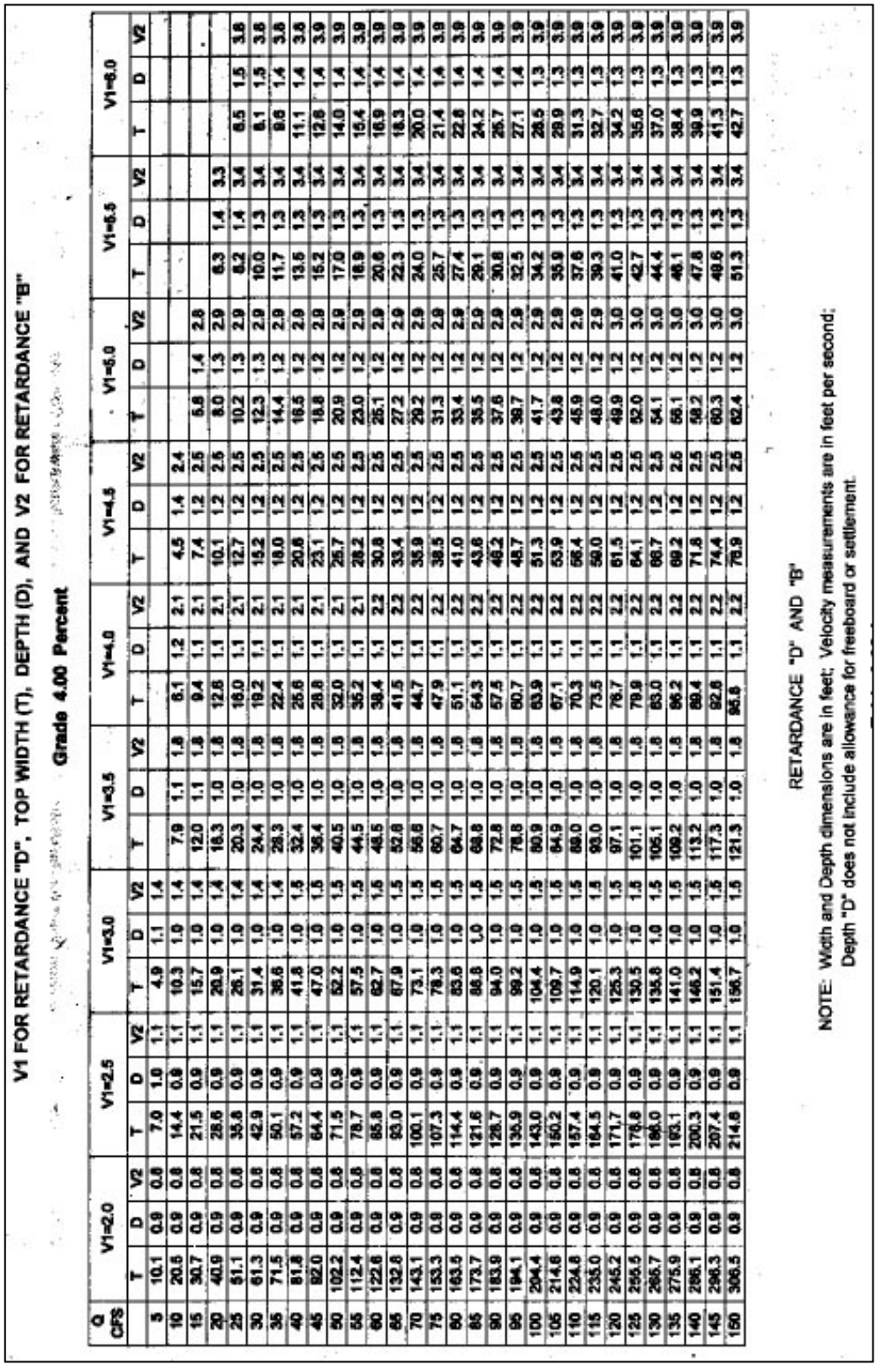
Grade 2.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	7.1	1.2	0.9																								
10	14.7	1.2	0.9	9.5	1.3	1.2	7.0	1.4	1.5																		
15	22.0	1.2	0.9	14.5	1.3	1.2	10.8	1.4	1.5	8.0	1.5	1.9	5.5	1.9	2.1												
20	29.3	1.2	0.9	19.6	1.2	1.2	14.6	1.3	1.5	10.9	1.5	1.9	8.1	1.8	2.3	5.5	2.1	2.6									
25	36.6	1.2	0.9	24.4	1.2	1.2	18.5	1.3	1.5	13.6	1.4	1.9	10.4	1.8	2.3	7.9	1.8	2.7									
30	43.9	1.2	0.9	29.3	1.2	1.2	22.2	1.3	1.6	16.6	1.4	1.9	12.7	1.5	2.3	9.7	1.7	2.7	7.3	2.0	3.1						
35	51.2	1.2	0.9	34.2	1.2	1.2	25.8	1.3	1.6	19.6	1.4	1.9	14.9	1.5	2.3	11.5	1.7	2.7	8.9	1.8	3.2						
40	58.5	1.2	0.9	39.0	1.2	1.2	29.5	1.3	1.6	22.4	1.4	1.9	17.1	1.5	2.3	13.3	1.6	2.8	10.4	1.8	3.2	8.0	2.1	3.6			
45	65.8	1.2	0.9	43.9	1.2	1.2	33.2	1.3	1.6	25.2	1.4	1.9	19.3	1.5	2.3	15.0	1.6	2.8	11.8	1.8	3.2	9.2	2.0	3.7			
50	73.1	1.2	0.9	48.8	1.2	1.2	36.8	1.3	1.6	28.0	1.4	1.9	21.7	1.5	2.3	16.7	1.6	2.8	13.2	1.8	3.2	10.5	1.9	3.7	7.9	2.3	4.1
55	80.4	1.2	0.9	53.6	1.2	1.2	40.5	1.3	1.6	30.7	1.4	1.9	23.9	1.5	2.3	18.5	1.6	2.8	14.6	1.7	3.2	11.7	1.9	3.7	9.2	2.2	4.1
60	87.7	1.2	0.9	58.5	1.2	1.2	44.2	1.3	1.6	33.5	1.4	1.9	26.0	1.5	2.3	20.2	1.6	2.8	16.0	1.7	3.2	12.8	1.9	3.7	10.2	2.1	4.1
65	95.0	1.2	0.9	63.4	1.2	1.2	47.8	1.3	1.6	36.3	1.4	1.9	28.2	1.5	2.3	22.1	1.6	2.8	17.4	1.7	3.3	14.0	1.9	3.7	11.3	2.1	4.2
70	102.3	1.2	0.9	68.2	1.2	1.2	51.6	1.3	1.6	39.1	1.4	1.9	30.3	1.5	2.3	23.8	1.6	2.8	18.8	1.7	3.3	15.2	1.9	3.7	12.3	2.1	4.2
75	109.6	1.2	0.9	73.1	1.2	1.2	55.2	1.3	1.6	41.9	1.4	1.9	32.5	1.5	2.3	25.5	1.6	2.8	20.1	1.7	3.3	16.2	1.8	3.7	13.2	2.0	4.2
80	116.9	1.2	0.9	78.0	1.2	1.2	58.9	1.3	1.6	44.7	1.4	1.9	34.6	1.5	2.3	27.2	1.6	2.8	21.5	1.7	3.3	17.4	1.8	3.6	14.2	2.0	4.2
85	124.2	1.2	0.9	82.9	1.2	1.2	62.6	1.3	1.6	47.4	1.4	1.9	36.8	1.5	2.3	28.9	1.6	2.8	22.9	1.7	3.3	18.6	1.8	3.8	16.1	2.0	4.2
90	131.5	1.2	0.9	87.7	1.2	1.2	66.3	1.3	1.6	50.2	1.4	1.9	39.0	1.5	2.3	30.6	1.6	2.8	24.6	1.7	3.2	19.6	1.8	3.8	18.1	2.0	4.2
95	138.8	1.2	0.9	92.6	1.2	1.2	69.9	1.3	1.6	53.0	1.4	1.9	41.1	1.5	2.3	32.3	1.6	2.8	25.9	1.7	3.3	20.8	1.8	3.8	17.0	2.0	4.2
100	146.1	1.2	0.9	97.5	1.2	1.2	73.6	1.3	1.6	55.8	1.4	1.9	43.3	1.5	2.3	34.0	1.6	2.8	27.3	1.7	3.3	21.9	1.8	3.8	18.0	2.0	4.2
105	153.4	1.2	0.9	102.3	1.2	1.2	77.3	1.3	1.6	58.6	1.4	1.9	45.4	1.5	2.3	35.7	1.6	2.8	28.6	1.7	3.3	23.0	1.8	3.8	18.9	2.0	4.2
110	160.7	1.2	0.9	107.2	1.2	1.2	81.0	1.3	1.6	61.4	1.4	1.9	47.6	1.5	2.3	37.3	1.6	2.8	30.0	1.7	3.3	24.1	1.8	3.8	19.8	2.0	4.2
115	168.0	1.2	0.9	112.1	1.2	1.2	84.7	1.3	1.6	64.2	1.4	1.9	49.8	1.5	2.3	39.0	1.6	2.8	31.3	1.7	3.3	25.3	1.8	3.8	20.8	2.0	4.2
120	175.3	1.2	0.9	117.0	1.2	1.2	88.3	1.3	1.6	67.0	1.4	1.9	51.9	1.5	2.3	40.7	1.6	2.8	32.7	1.7	3.3	26.7	1.8	3.7	21.7	2.0	4.2
125	182.6	1.2	0.9	121.8	1.2	1.2	92.0	1.3	1.6	69.7	1.4	1.9	54.1	1.5	2.3	42.4	1.6	2.8	34.1	1.7	3.3	27.8	1.8	3.7	22.6	1.9	4.3
130	189.9	1.2	0.9	126.7	1.2	1.2	95.7	1.3	1.6	72.5	1.4	1.9	56.2	1.5	2.3	44.1	1.6	2.8	35.4	1.7	3.3	28.9	1.8	3.7	23.6	1.9	4.3
135	197.3	1.2	0.9	131.6	1.2	1.2	99.4	1.3	1.6	75.3	1.4	1.9	58.4	1.5	2.3	45.8	1.6	2.8	36.8	1.7	3.3	30.0	1.8	3.7	24.5	1.9	4.3
140	204.6	1.2	0.9	136.5	1.2	1.2	103.1	1.3	1.6	78.1	1.4	1.9	60.6	1.5	2.3	47.5	1.6	2.8	38.1	1.7	3.3	31.1	1.8	3.7	25.4	1.9	4.3
145	211.9	1.2	0.9	141.3	1.2	1.2	106.7	1.3	1.6	80.9	1.4	1.9	62.7	1.5	2.3	49.2	1.6	2.8	39.5	1.7	3.3	32.3	1.8	3.7	26.4	1.9	4.3
150	219.2	1.2	0.9	146.2	1.2	1.2	110.4	1.3	1.6	83.7	1.4	1.9	64.9	1.5	2.3	50.9	1.6	2.8	40.8	1.7	3.3	33.4	1.8	3.7	27.3	1.9	4.3

RETARDANCE "D" AND "B"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-6 Parabolic Diversion Design Chart (Retardance “D” and “B”, Grade 4.00%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "B"

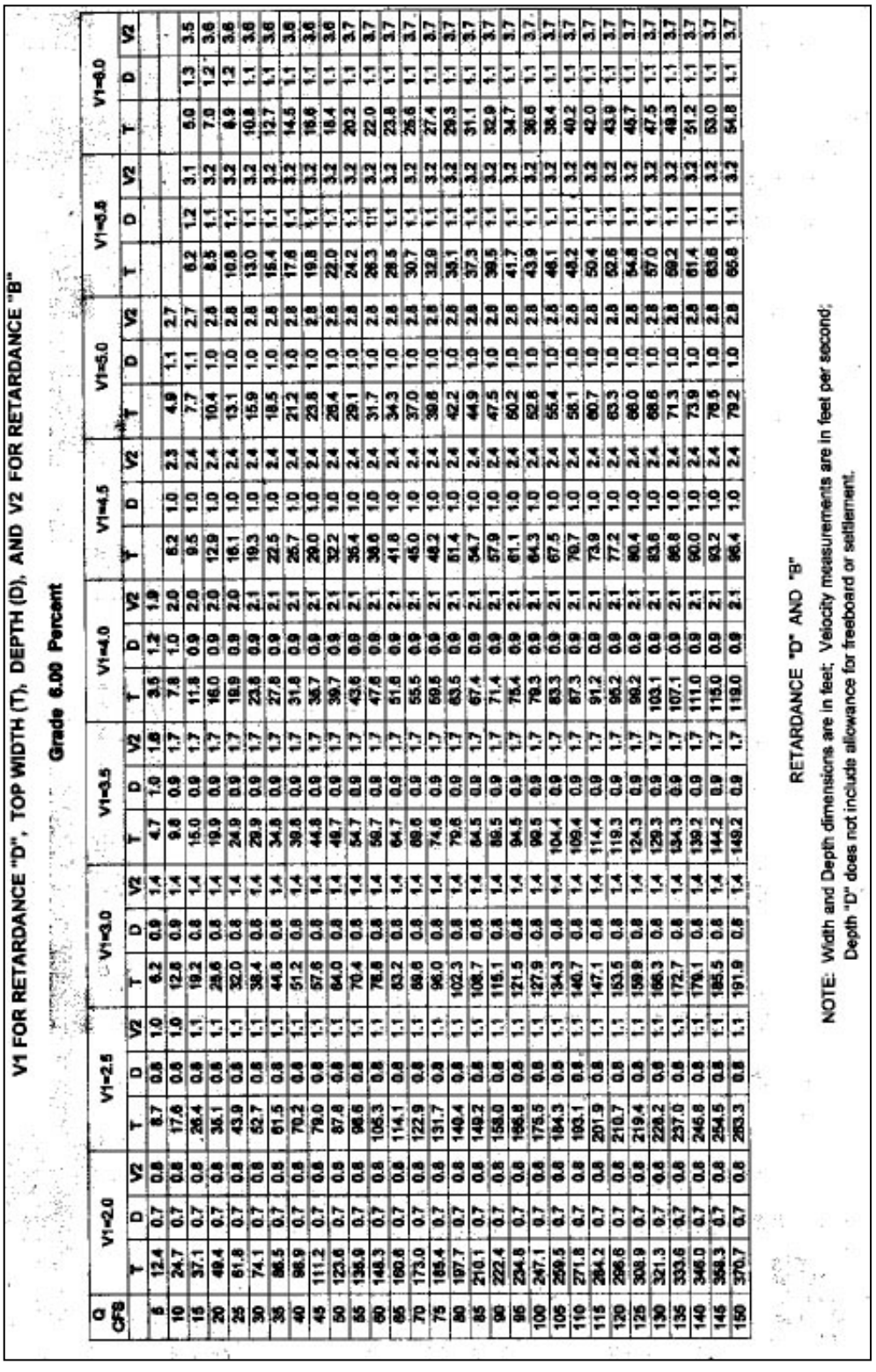
Grade 4.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	10.1	0.9	0.8	7.0	1.0	1.1	4.9	1.1	1.4																		
10	20.5	0.9	0.8	14.4	0.8	1.1	10.3	1.0	1.4	7.9	1.1	1.8	6.1	1.2	2.1	4.5	1.4	2.4									
15	30.7	0.9	0.8	21.5	0.9	1.1	15.7	1.0	1.4	12.0	1.1	1.8	9.4	1.1	2.1	7.4	1.2	2.5	5.8	1.4	2.8						
20	40.9	0.9	0.8	28.6	0.9	1.1	20.9	1.0	1.4	16.3	1.0	1.8	12.6	1.1	2.1	10.1	1.2	2.5	8.0	1.3	2.9	6.3	1.4	3.3			
25	51.1	0.9	0.8	35.8	0.9	1.1	26.1	1.0	1.4	20.3	1.0	1.8	16.0	1.1	2.1	12.7	1.2	2.5	10.2	1.3	2.9	8.2	1.4	3.4	6.5	1.5	3.8
30	61.3	0.9	0.8	42.9	0.9	1.1	31.4	1.0	1.4	24.4	1.0	1.8	19.2	1.1	2.1	15.2	1.2	2.5	12.3	1.3	2.9	10.0	1.3	3.4	8.1	1.5	3.8
35	71.5	0.9	0.8	50.1	0.9	1.1	36.6	1.0	1.4	28.3	1.0	1.8	22.4	1.1	2.1	18.0	1.2	2.5	14.4	1.2	2.9	11.7	1.3	3.4	9.6	1.4	3.8
40	81.8	0.9	0.8	57.2	0.9	1.1	41.8	1.0	1.5	32.4	1.0	1.8	25.6	1.1	2.1	20.6	1.2	2.5	16.5	1.2	2.9	13.6	1.3	3.4	11.1	1.4	3.8
45	92.0	0.9	0.8	64.4	0.9	1.1	47.0	1.0	1.5	36.4	1.0	1.8	28.8	1.1	2.1	23.1	1.2	2.5	18.8	1.2	2.9	15.2	1.3	3.4	12.6	1.4	3.9
50	102.2	0.9	0.8	71.5	0.9	1.1	52.2	1.0	1.5	40.5	1.0	1.8	32.0	1.1	2.1	25.7	1.2	2.5	20.9	1.2	2.9	17.0	1.3	3.4	14.0	1.4	3.9
55	112.4	0.9	0.8	78.7	0.9	1.1	57.5	1.0	1.5	44.5	1.0	1.8	35.2	1.1	2.1	28.2	1.2	2.5	23.0	1.2	2.9	18.9	1.3	3.4	15.4	1.4	3.9
60	122.6	0.9	0.8	85.8	0.9	1.1	62.7	1.0	1.5	48.5	1.0	1.8	38.4	1.1	2.2	30.8	1.2	2.5	25.1	1.2	2.9	20.6	1.3	3.4	16.9	1.4	3.9
65	132.8	0.9	0.8	93.0	0.9	1.1	67.9	1.0	1.5	52.8	1.0	1.8	41.5	1.1	2.2	33.4	1.2	2.5	27.2	1.2	2.9	22.3	1.3	3.4	18.3	1.4	3.9
70	143.1	0.9	0.8	100.1	0.9	1.1	73.1	1.0	1.5	56.8	1.0	1.8	44.7	1.1	2.2	35.9	1.2	2.5	29.2	1.2	2.9	24.0	1.3	3.4	20.0	1.4	3.9
75	153.3	0.8	0.8	107.3	0.9	1.1	78.3	1.0	1.5	60.7	1.0	1.8	47.9	1.1	2.2	38.5	1.2	2.5	31.3	1.2	2.9	25.7	1.3	3.4	21.4	1.4	3.9
80	163.6	0.9	0.8	114.4	0.9	1.1	83.6	1.0	1.5	64.7	1.0	1.8	51.1	1.1	2.2	41.0	1.2	2.5	33.4	1.2	2.9	27.4	1.3	3.4	22.8	1.4	3.9
85	173.7	0.9	0.8	121.6	0.9	1.1	88.8	1.0	1.5	68.8	1.0	1.8	54.3	1.1	2.2	43.6	1.2	2.5	35.5	1.2	2.9	29.1	1.3	3.4	24.2	1.4	3.9
90	183.9	0.9	0.8	128.7	0.9	1.1	94.0	1.0	1.5	72.8	1.0	1.8	57.5	1.1	2.2	46.2	1.2	2.5	37.6	1.2	2.9	30.8	1.3	3.4	25.7	1.4	3.9
95	194.1	0.9	0.8	135.9	0.9	1.1	99.2	1.0	1.5	76.8	1.0	1.8	60.7	1.1	2.2	48.7	1.2	2.5	39.7	1.2	2.9	32.5	1.3	3.4	27.1	1.4	3.9
100	204.4	0.9	0.8	143.0	0.9	1.1	104.4	1.0	1.5	80.9	1.0	1.8	63.9	1.1	2.2	51.3	1.2	2.5	41.7	1.2	2.9	34.2	1.3	3.4	28.5	1.3	3.9
105	214.6	0.9	0.8	150.2	0.9	1.1	109.7	1.0	1.5	84.9	1.0	1.8	67.1	1.1	2.2	53.9	1.2	2.5	43.8	1.2	2.9	35.9	1.3	3.4	29.9	1.3	3.9
110	224.8	0.9	0.8	157.4	0.9	1.1	114.9	1.0	1.5	88.9	1.0	1.8	70.3	1.1	2.2	56.4	1.2	2.5	45.9	1.2	2.9	37.6	1.3	3.4	31.3	1.3	3.9
115	235.0	0.9	0.8	164.5	0.9	1.1	120.1	1.0	1.5	93.0	1.0	1.8	73.5	1.1	2.2	59.0	1.2	2.5	48.0	1.2	2.9	39.3	1.3	3.4	32.7	1.3	3.9
120	245.2	0.9	0.8	171.7	0.9	1.1	125.3	1.0	1.5	97.1	1.0	1.8	76.7	1.1	2.2	61.5	1.2	2.5	49.9	1.2	3.0	41.0	1.3	3.4	34.2	1.3	3.9
125	255.5	0.9	0.8	178.8	0.9	1.1	130.5	1.0	1.5	101.1	1.0	1.8	79.9	1.1	2.2	64.1	1.2	2.5	52.0	1.2	3.0	42.7	1.3	3.4	35.6	1.3	3.9
130	265.7	0.9	0.8	186.0	0.9	1.1	135.8	1.0	1.5	105.1	1.0	1.8	83.0	1.1	2.2	66.7	1.2	2.5	54.1	1.2	3.0	44.4	1.3	3.4	37.0	1.3	3.9
135	275.9	0.9	0.8	193.1	0.9	1.1	141.0	1.0	1.5	109.2	1.0	1.8	86.2	1.1	2.2	69.2	1.2	2.5	56.1	1.2	3.0	46.1	1.3	3.4	38.4	1.3	3.9
140	286.1	0.9	0.8	200.3	0.9	1.1	146.2	1.0	1.5	113.2	1.0	1.8	89.4	1.1	2.2	71.8	1.2	2.5	58.2	1.2	3.0	47.8	1.3	3.4	39.9	1.3	3.9
145	296.3	0.9	0.8	207.4	0.9	1.1	151.4	1.0	1.5	117.3	1.0	1.8	92.6	1.1	2.2	74.4	1.2	2.5	60.3	1.2	3.0	49.6	1.3	3.4	41.3	1.3	3.9
150	306.5	0.9	0.8	214.6	0.9	1.1	156.7	1.0	1.5	121.3	1.0	1.8	95.8	1.1	2.2	76.9	1.2	2.5	62.4	1.2	3.0	51.3	1.3	3.4	42.7	1.3	3.9

RETARDANCE "D" AND "B"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-7 Parabolic Diversion Design Chart (Retardance “D” and “B”, Grade 6.00%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "B"

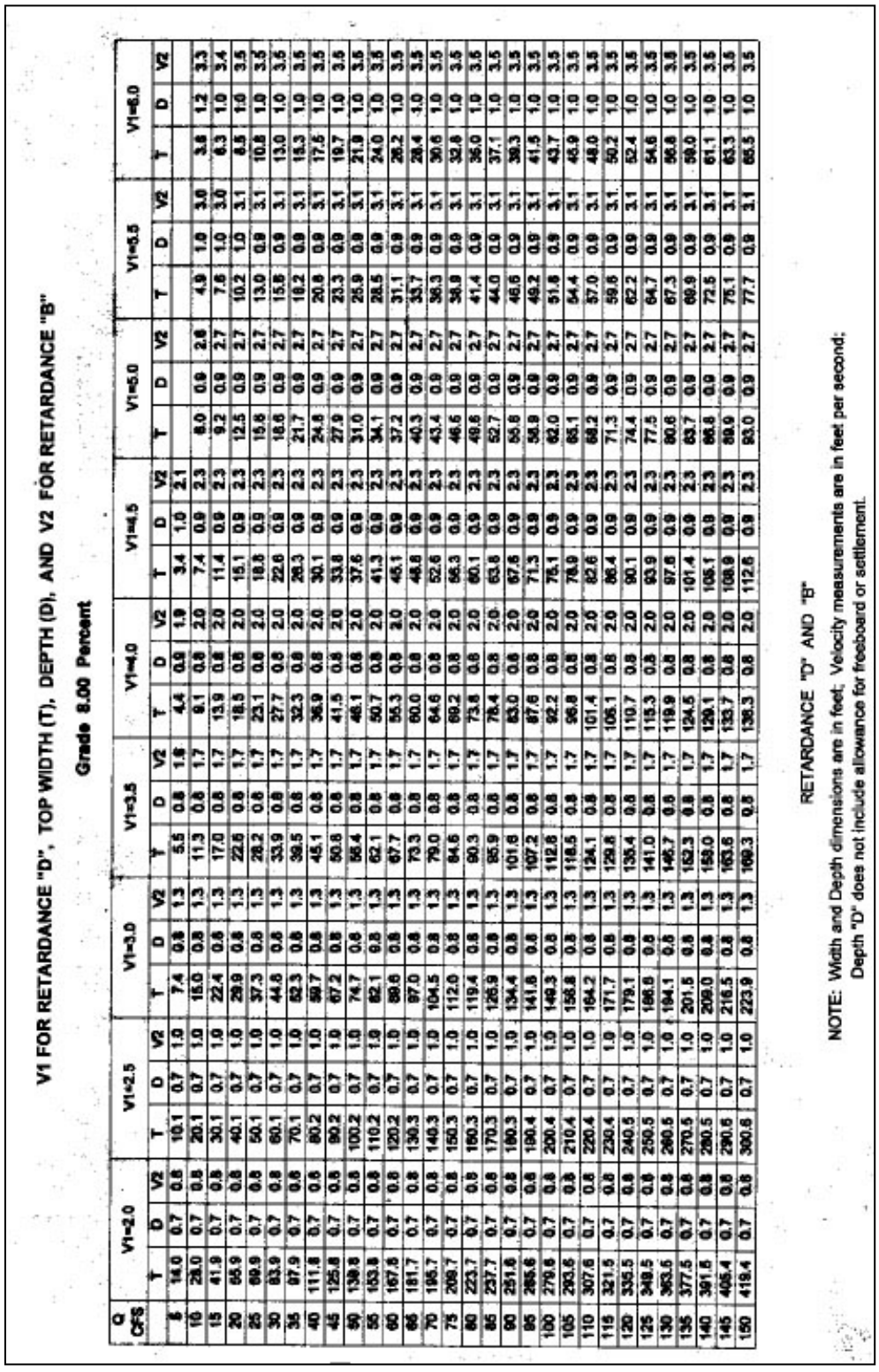
Grade 6.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	12.4	0.7	0.8	8.7	0.8	1.0	6.2	0.9	1.4	4.7	1.0	1.8	3.5	1.2	1.9												
10	24.7	0.7	0.8	17.6	0.8	1.0	12.6	0.9	1.4	9.8	0.9	1.7	7.8	1.0	2.0	6.2	1.0	2.3	4.8	1.1	2.7						
15	37.1	0.7	0.8	26.4	0.8	1.1	19.2	0.8	1.4	15.0	0.9	1.7	11.8	0.9	2.0	9.5	1.0	2.4	7.7	1.1	2.7	6.2	1.2	3.1	5.0	1.3	3.5
20	49.4	0.7	0.8	36.1	0.8	1.1	26.6	0.8	1.4	19.9	0.9	1.7	16.0	0.9	2.0	12.9	1.0	2.4	10.4	1.0	2.8	8.5	1.1	3.2	7.0	1.2	3.6
25	61.8	0.7	0.8	43.9	0.8	1.1	32.0	0.8	1.4	24.9	0.9	1.7	19.9	0.9	2.0	16.1	1.0	2.4	13.1	1.0	2.8	10.8	1.1	3.2	8.9	1.2	3.6
30	74.1	0.7	0.8	52.7	0.8	1.1	38.4	0.8	1.4	29.9	0.9	1.7	23.8	0.9	2.1	19.3	1.0	2.4	15.9	1.0	2.8	13.0	1.1	3.2	10.8	1.1	3.6
35	86.5	0.7	0.8	61.5	0.8	1.1	44.8	0.8	1.4	34.8	0.9	1.7	27.8	0.9	2.1	22.5	1.0	2.4	18.5	1.0	2.8	15.4	1.1	3.2	12.7	1.1	3.6
40	98.9	0.7	0.8	70.2	0.8	1.1	51.2	0.8	1.4	39.8	0.9	1.7	31.8	0.9	2.1	25.7	1.0	2.4	21.2	1.0	2.8	17.8	1.1	3.2	14.5	1.1	3.6
45	111.2	0.7	0.8	79.0	0.8	1.1	57.6	0.8	1.4	44.8	0.9	1.7	35.7	0.9	2.1	29.0	1.0	2.4	23.8	1.0	2.8	19.8	1.1	3.2	16.6	1.1	3.6
50	123.6	0.7	0.8	87.8	0.8	1.1	64.0	0.8	1.4	49.7	0.9	1.7	39.7	0.9	2.1	32.2	1.0	2.4	26.4	1.0	2.8	22.0	1.1	3.2	18.4	1.1	3.6
55	136.0	0.7	0.8	96.6	0.8	1.1	70.4	0.8	1.4	54.7	0.9	1.7	43.6	0.9	2.1	35.4	1.0	2.4	29.1	1.0	2.8	24.2	1.1	3.2	20.2	1.1	3.7
60	148.3	0.7	0.8	105.3	0.8	1.1	76.8	0.8	1.4	58.7	0.9	1.7	47.6	0.9	2.1	38.6	1.0	2.4	31.7	1.0	2.8	26.3	1.1	3.2	22.0	1.1	3.7
65	160.6	0.7	0.8	114.1	0.8	1.1	83.2	0.8	1.4	64.7	0.9	1.7	51.6	0.9	2.1	41.8	1.0	2.4	34.3	1.0	2.8	28.5	1.1	3.2	23.8	1.1	3.7
70	173.0	0.7	0.8	122.9	0.8	1.1	89.6	0.8	1.4	69.6	0.9	1.7	55.5	0.9	2.1	45.0	1.0	2.4	37.0	1.0	2.8	30.7	1.1	3.2	25.6	1.1	3.7
75	185.4	0.7	0.8	131.7	0.8	1.1	96.0	0.8	1.4	74.6	0.9	1.7	59.6	0.9	2.1	48.2	1.0	2.4	39.6	1.0	2.8	32.9	1.1	3.2	27.4	1.1	3.7
80	197.7	0.7	0.8	140.4	0.8	1.1	102.3	0.8	1.4	79.6	0.9	1.7	63.5	0.9	2.1	51.4	1.0	2.4	42.2	1.0	2.8	35.1	1.1	3.2	29.3	1.1	3.7
85	210.1	0.7	0.8	149.2	0.8	1.1	108.7	0.8	1.4	84.5	0.9	1.7	67.4	0.9	2.1	54.7	1.0	2.4	44.9	1.0	2.8	37.3	1.1	3.2	31.1	1.1	3.7
90	222.4	0.7	0.8	158.0	0.8	1.1	115.1	0.8	1.4	89.5	0.9	1.7	71.4	0.9	2.1	57.9	1.0	2.4	47.5	1.0	2.8	39.5	1.1	3.2	32.9	1.1	3.7
95	234.8	0.7	0.8	166.8	0.8	1.1	121.5	0.8	1.4	94.5	0.9	1.7	75.4	0.9	2.1	61.1	1.0	2.4	50.2	1.0	2.8	41.7	1.1	3.2	34.7	1.1	3.7
100	247.1	0.7	0.8	175.5	0.8	1.1	127.9	0.8	1.4	99.5	0.9	1.7	79.3	0.9	2.1	64.3	1.0	2.4	52.8	1.0	2.8	43.9	1.1	3.2	36.6	1.1	3.7
105	259.5	0.7	0.8	184.3	0.8	1.1	134.3	0.8	1.4	104.4	0.9	1.7	83.3	0.9	2.1	67.5	1.0	2.4	55.4	1.0	2.8	46.1	1.1	3.2	38.4	1.1	3.7
110	271.8	0.7	0.8	193.1	0.8	1.1	140.7	0.8	1.4	109.4	0.9	1.7	87.3	0.9	2.1	70.7	1.0	2.4	58.1	1.0	2.8	48.2	1.1	3.2	40.2	1.1	3.7
115	284.2	0.7	0.8	201.9	0.8	1.1	147.1	0.8	1.4	114.4	0.9	1.7	91.2	0.9	2.1	73.9	1.0	2.4	60.7	1.0	2.8	50.4	1.1	3.2	42.0	1.1	3.7
120	296.6	0.7	0.8	210.7	0.8	1.1	153.5	0.8	1.4	119.3	0.9	1.7	95.2	0.9	2.1	77.2	1.0	2.4	63.3	1.0	2.8	52.6	1.1	3.2	43.9	1.1	3.7
125	308.9	0.7	0.8	219.4	0.8	1.1	159.9	0.8	1.4	124.3	0.9	1.7	99.2	0.9	2.1	80.4	1.0	2.4	66.0	1.0	2.8	54.8	1.1	3.2	45.7	1.1	3.7
130	321.3	0.7	0.8	228.2	0.8	1.1	166.3	0.8	1.4	129.3	0.9	1.7	103.1	0.9	2.1	83.6	1.0	2.4	68.6	1.0	2.8	57.0	1.1	3.2	47.5	1.1	3.7
135	333.6	0.7	0.8	237.0	0.8	1.1	172.7	0.8	1.4	134.3	0.9	1.7	107.1	0.9	2.1	86.8	1.0	2.4	71.3	1.0	2.8	59.2	1.1	3.2	49.3	1.1	3.7
140	346.0	0.7	0.8	245.8	0.8	1.1	179.1	0.8	1.4	139.2	0.9	1.7	111.0	0.9	2.1	90.0	1.0	2.4	73.9	1.0	2.8	61.4	1.1	3.2	51.2	1.1	3.7
145	358.3	0.7	0.8	254.5	0.8	1.1	185.5	0.8	1.4	144.2	0.9	1.7	115.0	0.9	2.1	93.2	1.0	2.4	76.5	1.0	2.8	63.6	1.1	3.2	53.0	1.1	3.7
150	370.7	0.7	0.8	263.3	0.8	1.1	191.9	0.8	1.4	149.2	0.9	1.7	119.0	0.9	2.1	96.4	1.0	2.4	79.2	1.0	2.8	66.8	1.1	3.2	54.8	1.1	3.7

RETARDANCE "D" AND "B"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-8 Parabolic Diversion Design Chart (Retardance “D” and “B”, Grade 8.00%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "B"

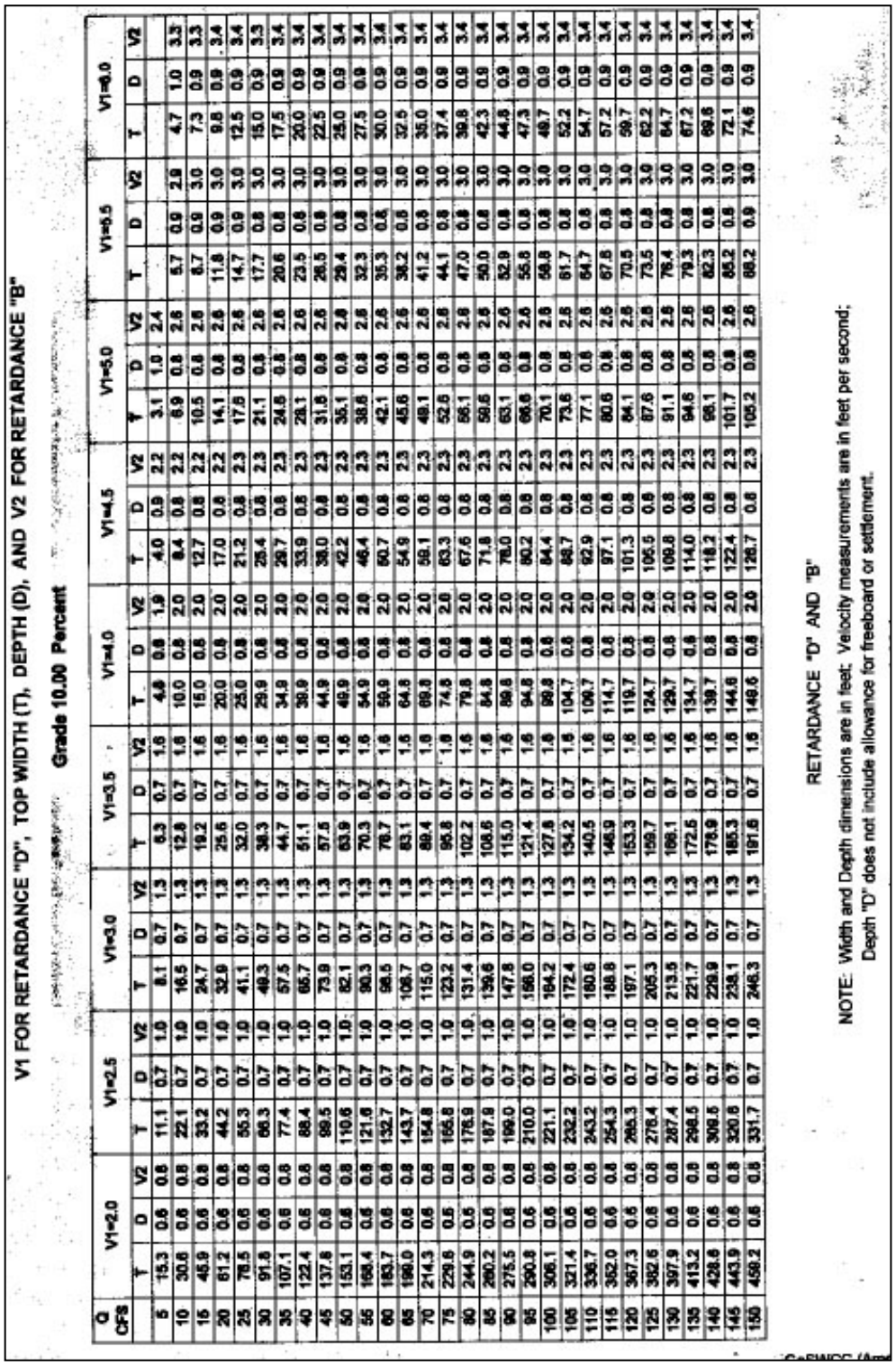
Grade 8.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	14.0	0.7	0.8	10.1	0.7	1.0	7.4	0.8	1.3	5.5	0.8	1.6	4.4	0.8	1.9	3.4	1.0	2.1									
10	28.0	0.7	0.8	20.1	0.7	1.0	15.0	0.8	1.3	11.3	0.8	1.7	9.1	0.8	2.0	7.4	0.9	2.3	8.0	0.9	2.6	4.9	1.0	3.0	3.6	1.2	3.3
15	41.9	0.7	0.8	30.1	0.7	1.0	22.4	0.8	1.3	17.0	0.8	1.7	13.9	0.8	2.0	11.4	0.9	2.3	9.2	0.9	2.7	7.6	1.0	3.0	6.3	1.0	3.4
20	55.9	0.7	0.8	40.1	0.7	1.0	29.9	0.8	1.3	22.6	0.8	1.7	18.5	0.8	2.0	15.1	0.9	2.3	12.5	0.9	2.7	10.2	1.0	3.1	8.5	1.0	3.5
25	69.9	0.7	0.8	50.1	0.7	1.0	37.3	0.8	1.3	28.2	0.8	1.7	23.1	0.8	2.0	18.8	0.9	2.3	15.6	0.9	2.7	13.0	0.9	3.1	10.8	1.0	3.5
30	83.9	0.7	0.8	60.1	0.7	1.0	44.8	0.8	1.3	33.9	0.8	1.7	27.7	0.8	2.0	22.6	0.8	2.3	18.8	0.9	2.7	15.6	0.9	3.1	13.0	1.0	3.5
35	97.9	0.7	0.8	70.1	0.7	1.0	52.3	0.8	1.3	39.5	0.8	1.7	32.3	0.8	2.0	26.3	0.9	2.3	21.7	0.9	2.7	18.2	0.9	3.1	15.3	1.0	3.5
40	111.8	0.7	0.8	80.2	0.7	1.0	59.7	0.8	1.3	45.1	0.8	1.7	36.9	0.8	2.0	30.1	0.9	2.3	24.8	0.9	2.7	20.8	0.9	3.1	17.5	1.0	3.5
45	125.8	0.7	0.8	90.2	0.7	1.0	67.2	0.8	1.3	50.8	0.8	1.7	41.5	0.8	2.0	33.8	0.9	2.3	27.9	0.9	2.7	23.3	0.9	3.1	19.7	1.0	3.5
50	139.8	0.7	0.8	100.2	0.7	1.0	74.7	0.8	1.3	56.4	0.8	1.7	46.1	0.8	2.0	37.6	0.9	2.3	31.0	0.9	2.7	25.9	0.9	3.1	21.9	1.0	3.5
55	153.8	0.7	0.8	110.2	0.7	1.0	82.1	0.8	1.3	62.1	0.8	1.7	50.7	0.8	2.0	41.3	0.9	2.3	34.1	0.9	2.7	28.5	0.9	3.1	24.0	1.0	3.5
60	167.8	0.7	0.8	120.2	0.7	1.0	89.6	0.8	1.3	67.7	0.8	1.7	55.3	0.8	2.0	45.1	0.9	2.3	37.2	0.9	2.7	31.1	0.9	3.1	26.2	1.0	3.5
65	181.7	0.7	0.8	130.3	0.7	1.0	97.0	0.8	1.3	73.3	0.8	1.7	60.0	0.8	2.0	48.8	0.9	2.3	40.3	0.9	2.7	33.7	0.9	3.1	28.4	1.0	3.5
70	195.7	0.7	0.8	140.3	0.7	1.0	104.5	0.8	1.3	79.0	0.8	1.7	64.6	0.8	2.0	52.6	0.9	2.3	43.4	0.9	2.7	36.3	0.9	3.1	30.6	1.0	3.5
75	209.7	0.7	0.8	150.3	0.7	1.0	112.0	0.8	1.3	84.6	0.8	1.7	69.2	0.8	2.0	56.3	0.9	2.3	46.6	0.9	2.7	38.9	0.9	3.1	32.8	1.0	3.5
80	223.7	0.7	0.8	160.3	0.7	1.0	119.4	0.8	1.3	90.3	0.8	1.7	73.8	0.8	2.0	60.1	0.9	2.3	49.8	0.9	2.7	41.4	0.9	3.1	35.0	1.0	3.5
85	237.7	0.7	0.8	170.3	0.7	1.0	126.9	0.8	1.3	95.9	0.8	1.7	78.4	0.8	2.0	63.8	0.9	2.3	52.7	0.9	2.7	44.0	0.9	3.1	37.1	1.0	3.5
90	251.6	0.7	0.8	180.3	0.7	1.0	134.4	0.8	1.3	101.8	0.8	1.7	83.0	0.8	2.0	67.6	0.9	2.3	55.8	0.9	2.7	46.6	0.9	3.1	39.3	1.0	3.5
95	265.6	0.7	0.8	190.4	0.7	1.0	141.8	0.8	1.3	107.2	0.8	1.7	87.6	0.8	2.0	71.3	0.9	2.3	58.9	0.9	2.7	49.2	0.9	3.1	41.5	1.0	3.5
100	279.6	0.7	0.8	200.4	0.7	1.0	149.3	0.8	1.3	112.8	0.8	1.7	92.2	0.8	2.0	75.1	0.9	2.3	62.0	0.9	2.7	51.8	0.9	3.1	43.7	1.0	3.5
105	293.6	0.7	0.8	210.4	0.7	1.0	156.8	0.8	1.3	118.5	0.8	1.7	96.8	0.8	2.0	78.9	0.9	2.3	65.1	0.9	2.7	54.4	0.9	3.1	45.9	1.0	3.5
110	307.6	0.7	0.8	220.4	0.7	1.0	164.2	0.8	1.3	124.1	0.8	1.7	101.4	0.8	2.0	82.6	0.9	2.3	68.2	0.9	2.7	57.0	0.9	3.1	48.0	1.0	3.5
115	321.6	0.7	0.8	230.4	0.7	1.0	171.7	0.8	1.3	129.8	0.8	1.7	106.1	0.8	2.0	86.4	0.9	2.3	71.3	0.9	2.7	59.6	0.9	3.1	50.2	1.0	3.5
120	335.5	0.7	0.8	240.5	0.7	1.0	179.1	0.8	1.3	135.4	0.8	1.7	110.7	0.8	2.0	90.1	0.9	2.3	74.4	0.9	2.7	62.2	0.9	3.1	52.4	1.0	3.5
125	349.5	0.7	0.8	250.5	0.7	1.0	186.6	0.8	1.3	141.0	0.8	1.7	115.3	0.8	2.0	93.9	0.9	2.3	77.5	0.9	2.7	64.7	0.9	3.1	54.6	1.0	3.5
130	363.5	0.7	0.8	260.5	0.7	1.0	194.1	0.8	1.3	146.7	0.8	1.7	119.9	0.8	2.0	97.6	0.9	2.3	80.6	0.9	2.7	67.3	0.9	3.1	56.8	1.0	3.5
135	377.5	0.7	0.8	270.5	0.7	1.0	201.6	0.8	1.3	152.3	0.8	1.7	124.5	0.8	2.0	101.4	0.9	2.3	83.7	0.9	2.7	69.9	0.9	3.1	59.0	1.0	3.5
140	391.5	0.7	0.8	280.5	0.7	1.0	209.0	0.8	1.3	158.0	0.8	1.7	129.1	0.8	2.0	105.1	0.9	2.3	86.8	0.9	2.7	72.5	0.9	3.1	61.1	1.0	3.5
145	405.4	0.7	0.8	290.6	0.7	1.0	216.5	0.8	1.3	163.6	0.8	1.7	133.7	0.8	2.0	108.9	0.9	2.3	89.9	0.9	2.7	75.1	0.9	3.1	63.3	1.0	3.5
150	419.4	0.7	0.8	300.6	0.7	1.0	223.9	0.8	1.3	169.3	0.8	1.7	138.3	0.8	2.0	112.6	0.9	2.3	93.0	0.9	2.7	77.7	0.9	3.1	65.5	1.0	3.5

RETARDANCE "D" AND "B"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-9 Parabolic Diversion Design Chart (Retardance “D” and “B”, Grade 10.00%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "B"

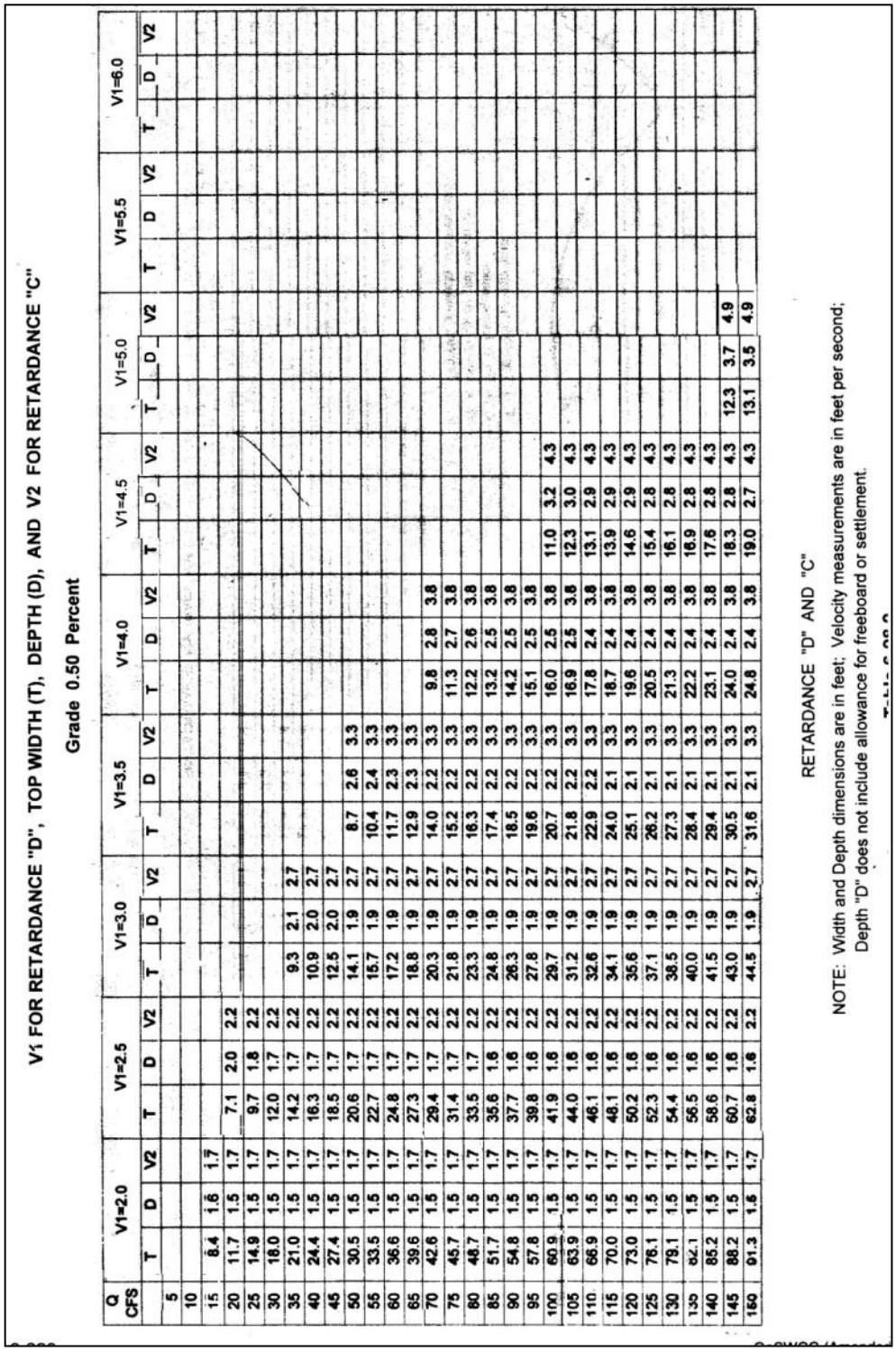
Grade 10.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=6.5			V1=8.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	15.3	0.6	0.8	11.1	0.7	1.0	8.1	0.7	1.3	6.3	0.7	1.6	4.8	0.8	1.9	4.0	0.9	2.2	3.1	1.0	2.4						
10	30.6	0.6	0.8	22.1	0.7	1.0	16.5	0.7	1.3	12.8	0.7	1.6	10.0	0.8	2.0	8.4	0.8	2.2	6.9	0.8	2.6	5.7	0.9	2.8	4.7	1.0	3.3
15	45.9	0.6	0.8	33.2	0.7	1.0	24.7	0.7	1.3	19.2	0.7	1.6	15.0	0.8	2.0	12.7	0.8	2.2	10.5	0.8	2.6	8.7	0.9	3.0	7.3	0.9	3.3
20	61.2	0.6	0.8	44.2	0.7	1.0	32.9	0.7	1.3	25.6	0.7	1.6	20.0	0.8	2.0	17.0	0.8	2.2	14.1	0.8	2.6	11.8	0.9	3.0	9.8	0.9	3.4
25	76.5	0.6	0.8	55.3	0.7	1.0	41.1	0.7	1.3	32.0	0.7	1.6	25.0	0.8	2.0	21.2	0.8	2.3	17.6	0.8	2.6	14.7	0.9	3.0	12.5	0.9	3.4
30	91.8	0.6	0.8	66.3	0.7	1.0	49.3	0.7	1.3	38.3	0.7	1.6	29.9	0.8	2.0	25.4	0.8	2.3	21.1	0.8	2.6	17.7	0.8	3.0	15.0	0.9	3.3
35	107.1	0.6	0.8	77.4	0.7	1.0	57.5	0.7	1.3	44.7	0.7	1.6	34.8	0.8	2.0	29.7	0.8	2.3	24.6	0.8	2.6	20.6	0.8	3.0	17.5	0.9	3.4
40	122.4	0.6	0.8	88.4	0.7	1.0	65.7	0.7	1.3	51.1	0.7	1.6	39.9	0.8	2.0	33.9	0.8	2.3	28.1	0.8	2.6	23.5	0.8	3.0	20.0	0.9	3.4
45	137.8	0.6	0.8	99.5	0.7	1.0	73.9	0.7	1.3	57.5	0.7	1.6	44.9	0.8	2.0	38.0	0.8	2.3	31.6	0.8	2.6	26.5	0.8	3.0	22.5	0.9	3.4
50	153.1	0.6	0.8	110.6	0.7	1.0	82.1	0.7	1.3	63.9	0.7	1.6	49.9	0.8	2.0	42.2	0.8	2.3	35.1	0.8	2.6	29.4	0.8	3.0	25.0	0.9	3.4
55	168.4	0.6	0.8	121.6	0.7	1.0	90.3	0.7	1.3	70.3	0.7	1.6	54.9	0.8	2.0	46.4	0.8	2.3	38.6	0.8	2.6	32.3	0.8	3.0	27.5	0.8	3.4
60	183.7	0.6	0.8	132.7	0.7	1.0	98.6	0.7	1.3	76.7	0.7	1.6	59.9	0.8	2.0	50.7	0.8	2.3	42.1	0.8	2.6	35.3	0.8	3.0	30.0	0.9	3.4
65	199.0	0.6	0.8	143.7	0.7	1.0	106.7	0.7	1.3	83.1	0.7	1.6	64.8	0.8	2.0	54.9	0.8	2.3	45.6	0.8	2.6	38.2	0.8	3.0	32.5	0.9	3.4
70	214.3	0.6	0.8	154.8	0.7	1.0	115.0	0.7	1.3	89.4	0.7	1.6	69.8	0.8	2.0	59.1	0.8	2.3	49.1	0.8	2.6	41.2	0.8	3.0	35.0	0.9	3.4
75	229.6	0.6	0.8	165.8	0.7	1.0	123.2	0.7	1.3	95.8	0.7	1.6	74.8	0.8	2.0	63.3	0.8	2.3	52.6	0.8	2.6	44.1	0.8	3.0	37.4	0.9	3.4
80	244.9	0.6	0.8	176.9	0.7	1.0	131.4	0.7	1.3	102.2	0.7	1.6	79.8	0.8	2.0	67.6	0.8	2.3	56.1	0.8	2.6	47.0	0.8	3.0	39.8	0.9	3.4
85	260.2	0.6	0.8	187.9	0.7	1.0	139.6	0.7	1.3	108.6	0.7	1.6	84.8	0.8	2.0	71.8	0.8	2.3	59.6	0.8	2.6	50.0	0.8	3.0	42.3	0.9	3.4
90	275.5	0.6	0.8	199.0	0.7	1.0	147.8	0.7	1.3	115.0	0.7	1.6	89.8	0.8	2.0	76.0	0.8	2.3	63.1	0.8	2.6	52.9	0.8	3.0	44.6	0.9	3.4
95	290.8	0.6	0.8	210.0	0.7	1.0	156.0	0.7	1.3	121.4	0.7	1.6	94.8	0.8	2.0	80.2	0.8	2.3	66.6	0.8	2.6	55.8	0.8	3.0	47.3	0.9	3.4
100	306.1	0.6	0.8	221.1	0.7	1.0	164.2	0.7	1.3	127.8	0.7	1.6	99.8	0.8	2.0	84.4	0.8	2.3	70.1	0.8	2.6	58.8	0.8	3.0	49.7	0.9	3.4
105	321.4	0.6	0.8	232.2	0.7	1.0	172.4	0.7	1.3	134.2	0.7	1.6	104.7	0.8	2.0	88.7	0.8	2.3	73.6	0.8	2.6	61.7	0.8	3.0	52.2	0.9	3.4
110	336.7	0.6	0.8	243.2	0.7	1.0	180.6	0.7	1.3	140.5	0.7	1.6	109.7	0.8	2.0	92.9	0.8	2.3	77.1	0.8	2.6	64.7	0.8	3.0	54.7	0.9	3.4
115	352.0	0.6	0.8	254.3	0.7	1.0	188.8	0.7	1.3	146.9	0.7	1.6	114.7	0.8	2.0	97.1	0.8	2.3	80.6	0.8	2.6	67.6	0.8	3.0	57.2	0.9	3.4
120	367.3	0.6	0.8	265.3	0.7	1.0	197.1	0.7	1.3	153.3	0.7	1.6	119.7	0.8	2.0	101.3	0.8	2.3	84.1	0.8	2.6	70.5	0.8	3.0	59.7	0.9	3.4
125	382.6	0.6	0.8	276.4	0.7	1.0	205.3	0.7	1.3	159.7	0.7	1.6	124.7	0.8	2.0	105.5	0.8	2.3	87.6	0.8	2.6	73.5	0.8	3.0	62.2	0.9	3.4
130	397.9	0.6	0.8	287.4	0.7	1.0	213.5	0.7	1.3	166.1	0.7	1.6	129.7	0.8	2.0	109.8	0.8	2.3	91.1	0.8	2.6	76.4	0.8	3.0	64.7	0.9	3.4
135	413.2	0.6	0.8	298.5	0.7	1.0	221.7	0.7	1.3	172.5	0.7	1.6	134.7	0.8	2.0	114.0	0.8	2.3	94.6	0.8	2.6	79.3	0.8	3.0	67.2	0.9	3.4
140	428.6	0.6	0.8	309.6	0.7	1.0	229.9	0.7	1.3	178.9	0.7	1.6	139.7	0.8	2.0	118.2	0.8	2.3	98.1	0.8	2.6	82.3	0.8	3.0	69.6	0.9	3.4
145	443.9	0.6	0.8	320.6	0.7	1.0	238.1	0.7	1.3	185.3	0.7	1.6	144.6	0.8	2.0	122.4	0.8	2.3	101.7	0.8	2.6	85.2	0.8	3.0	72.1	0.9	3.4
150	459.2	0.6	0.8	331.7	0.7	1.0	246.3	0.7	1.3	191.6	0.7	1.6	149.6	0.8	2.0	126.7	0.8	2.3	105.2	0.8	2.6	88.2	0.9	3.0	74.6	0.9	3.4

RETARDANCE "D" AND "B"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-10 Parabolic Diversion Design Chart (Retardance “D” and “C”, Grade .50%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "C"

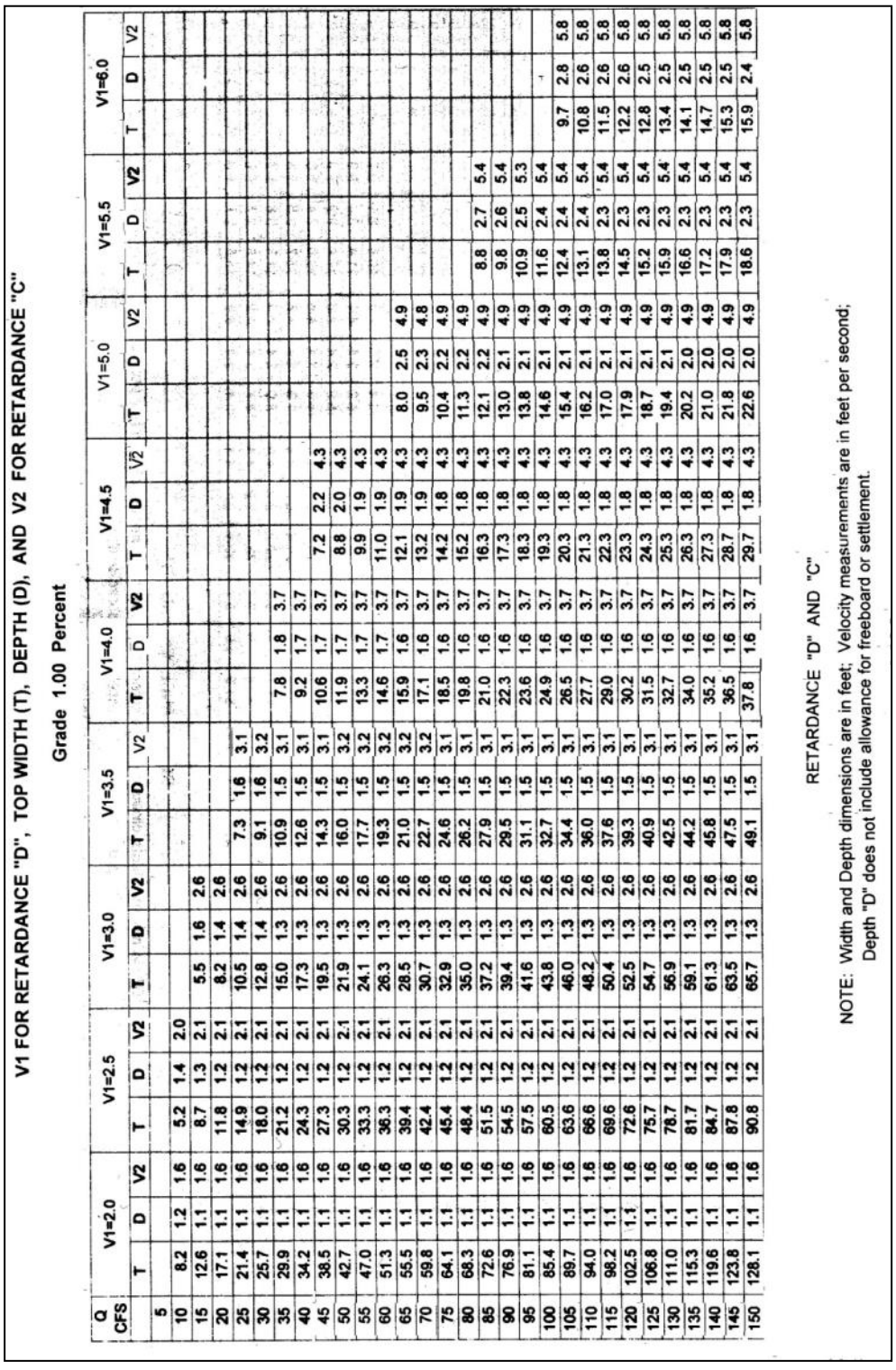
Grade 0.50 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5																											
10																											
15	8.4	1.6	1.7																								
20	11.7	1.5	1.7	7.1	2.0	2.2																					
25	14.9	1.5	1.7	9.7	1.8	2.2																					
30	18.0	1.5	1.7	12.0	1.7	2.2																					
35	21.0	1.5	1.7	14.2	1.7	2.2	9.3	2.1	2.7																		
40	24.4	1.5	1.7	16.3	1.7	2.2	10.9	2.0	2.7																		
45	27.4	1.5	1.7	18.5	1.7	2.2	12.5	2.0	2.7																		
50	30.5	1.5	1.7	20.6	1.7	2.2	14.1	1.9	2.7	8.7	2.6	3.3															
55	33.5	1.5	1.7	22.7	1.7	2.2	15.7	1.9	2.7	10.4	2.4	3.3															
60	36.6	1.5	1.7	24.8	1.7	2.2	17.2	1.9	2.7	11.7	2.3	3.3															
65	39.6	1.5	1.7	27.3	1.7	2.2	18.8	1.9	2.7	12.9	2.3	3.3															
70	42.6	1.5	1.7	29.4	1.7	2.2	20.3	1.9	2.7	14.0	2.2	3.3	9.8	2.8	3.8												
75	45.7	1.5	1.7	31.4	1.7	2.2	21.8	1.9	2.7	15.2	2.2	3.3	11.3	2.7	3.8												
80	48.7	1.5	1.7	33.5	1.7	2.2	23.3	1.9	2.7	16.3	2.2	3.3	12.2	2.6	3.8												
85	51.7	1.5	1.7	35.6	1.6	2.2	24.8	1.9	2.7	17.4	2.2	3.3	13.2	2.5	3.8												
90	54.8	1.5	1.7	37.7	1.6	2.2	26.3	1.9	2.7	18.5	2.2	3.3	14.2	2.5	3.8												
95	57.8	1.5	1.7	39.8	1.6	2.2	27.8	1.9	2.7	19.6	2.2	3.3	15.1	2.5	3.8												
100	60.9	1.5	1.7	41.9	1.6	2.2	29.7	1.9	2.7	20.7	2.2	3.3	16.0	2.5	3.8	11.0	3.2	4.3									
105	63.9	1.5	1.7	44.0	1.6	2.2	31.2	1.9	2.7	21.8	2.2	3.3	16.9	2.5	3.8	12.3	3.0	4.3									
110	66.9	1.5	1.7	46.1	1.6	2.2	32.6	1.9	2.7	22.9	2.2	3.3	17.8	2.4	3.8	13.1	2.9	4.3									
115	70.0	1.5	1.7	48.1	1.6	2.2	34.1	1.9	2.7	24.0	2.1	3.3	18.7	2.4	3.8	13.9	2.9	4.3									
120	73.0	1.5	1.7	50.2	1.6	2.2	35.6	1.9	2.7	25.1	2.1	3.3	19.6	2.4	3.8	14.6	2.9	4.3									
125	76.1	1.5	1.7	52.3	1.6	2.2	37.1	1.9	2.7	26.2	2.1	3.3	20.5	2.4	3.8	15.4	2.8	4.3									
130	79.1	1.5	1.7	54.4	1.6	2.2	38.5	1.9	2.7	27.3	2.1	3.3	21.3	2.4	3.8	16.1	2.8	4.3									
135	82.1	1.5	1.7	56.5	1.6	2.2	40.0	1.9	2.7	28.4	2.1	3.3	22.2	2.4	3.8	16.9	2.8	4.3									
140	85.2	1.5	1.7	58.6	1.6	2.2	41.5	1.9	2.7	29.4	2.1	3.3	23.1	2.4	3.8	17.6	2.8	4.3									
145	88.2	1.5	1.7	60.7	1.6	2.2	43.0	1.9	2.7	30.5	2.1	3.3	24.0	2.4	3.8	18.3	2.8	4.3	12.3	3.7	4.9						
150	91.3	1.6	1.7	62.8	1.6	2.2	44.5	1.9	2.7	31.6	2.1	3.3	24.8	2.4	3.8	19.0	2.7	4.3	13.1	3.5	4.9						

RETARDANCE "D" AND "C"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-11 Parabolic Diversion Design Chart (Retardance “D” and “C”, Grade 1.00%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "C"

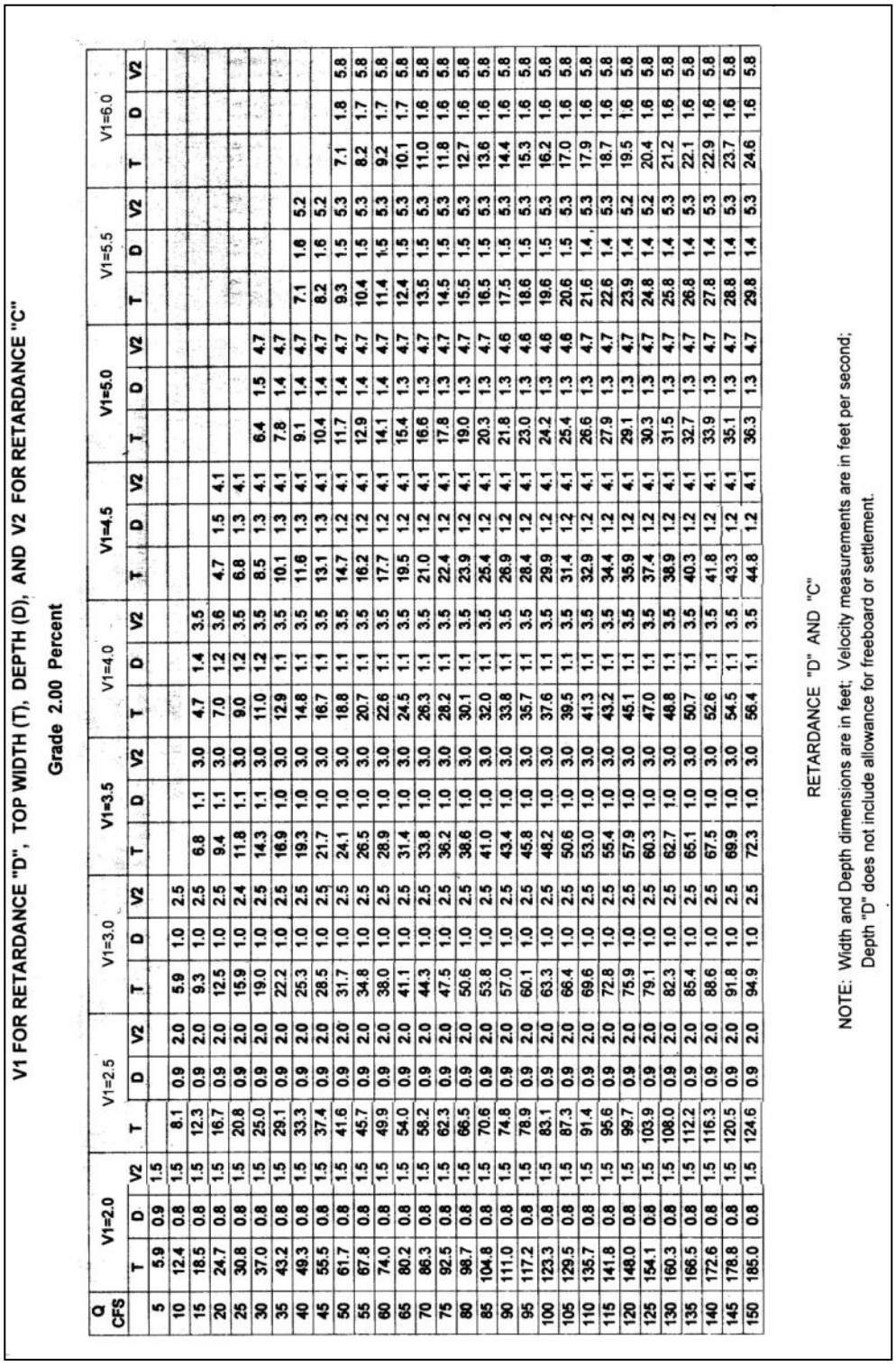
Grade 1.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5																											
10	8.2	1.2	1.6	5.2	1.4	2.0																					
15	12.6	1.1	1.6	8.7	1.3	2.1	5.5	1.6	2.6																		
20	17.1	1.1	1.6	11.8	1.2	2.1	8.2	1.4	2.6																		
25	21.4	1.1	1.6	14.9	1.2	2.1	10.5	1.4	2.6	7.3	1.6	3.1															
30	25.7	1.1	1.6	18.0	1.2	2.1	12.8	1.4	2.6	9.1	1.6	3.2															
35	29.9	1.1	1.6	21.2	1.2	2.1	15.0	1.3	2.6	10.9	1.5	3.1	7.8	1.8	3.7												
40	34.2	1.1	1.6	24.3	1.2	2.1	17.3	1.3	2.6	12.6	1.5	3.1	9.2	1.7	3.7												
45	38.5	1.1	1.6	27.3	1.2	2.1	19.5	1.3	2.6	14.3	1.5	3.1	10.6	1.7	3.7	7.2	2.2	4.3									
50	42.7	1.1	1.6	30.3	1.2	2.1	21.9	1.3	2.6	16.0	1.5	3.2	11.9	1.7	3.7	8.8	2.0	4.3									
55	47.0	1.1	1.6	33.3	1.2	2.1	24.1	1.3	2.6	17.7	1.5	3.2	13.3	1.7	3.7	9.9	1.9	4.3									
60	51.3	1.1	1.6	36.3	1.2	2.1	26.3	1.3	2.6	19.3	1.5	3.2	14.6	1.7	3.7	11.0	1.9	4.3									
65	55.5	1.1	1.6	39.4	1.2	2.1	28.5	1.3	2.6	21.0	1.5	3.2	15.9	1.6	3.7	12.1	1.9	4.3	8.0	2.5	4.9						
70	59.8	1.1	1.6	42.4	1.2	2.1	30.7	1.3	2.6	22.7	1.5	3.2	17.1	1.6	3.7	13.2	1.9	4.3	9.5	2.3	4.8						
75	64.1	1.1	1.6	45.4	1.2	2.1	32.9	1.3	2.6	24.6	1.5	3.1	18.5	1.6	3.7	14.2	1.8	4.3	10.4	2.2	4.9						
80	68.3	1.1	1.6	48.4	1.2	2.1	35.0	1.3	2.6	26.2	1.5	3.1	19.8	1.6	3.7	15.2	1.8	4.3	11.3	2.2	4.9						
85	72.6	1.1	1.6	51.5	1.2	2.1	37.2	1.3	2.6	27.9	1.5	3.1	21.0	1.6	3.7	16.3	1.8	4.3	12.1	2.2	4.9	8.8	2.7	5.4			
90	76.9	1.1	1.6	54.5	1.2	2.1	39.4	1.3	2.6	29.5	1.5	3.1	22.3	1.6	3.7	17.3	1.8	4.3	13.0	2.1	4.9	9.8	2.6	5.4			
95	81.1	1.1	1.6	57.5	1.2	2.1	41.6	1.3	2.6	31.1	1.5	3.1	23.6	1.6	3.7	18.3	1.8	4.3	13.8	2.1	4.9	10.9	2.5	5.3			
100	85.4	1.1	1.6	60.5	1.2	2.1	43.8	1.3	2.6	32.7	1.5	3.1	24.9	1.6	3.7	19.3	1.8	4.3	14.6	2.1	4.9	11.6	2.4	5.4			
105	89.7	1.1	1.6	63.6	1.2	2.1	46.0	1.3	2.6	34.4	1.5	3.1	26.5	1.6	3.7	20.3	1.8	4.3	15.4	2.1	4.9	12.4	2.4	5.4	9.7	2.8	5.8
110	94.0	1.1	1.6	66.6	1.2	2.1	48.2	1.3	2.6	36.0	1.5	3.1	27.7	1.6	3.7	21.3	1.8	4.3	16.2	2.1	4.9	13.1	2.4	5.4	10.8	2.6	5.8
115	98.2	1.1	1.6	69.6	1.2	2.1	50.4	1.3	2.6	37.6	1.5	3.1	29.0	1.6	3.7	22.3	1.8	4.3	17.0	2.1	4.9	13.8	2.3	5.4	11.5	2.6	5.8
120	102.5	1.1	1.6	72.6	1.2	2.1	52.5	1.3	2.6	39.3	1.5	3.1	30.2	1.6	3.7	23.3	1.8	4.3	17.9	2.1	4.9	14.5	2.3	5.4	12.2	2.6	5.8
125	106.8	1.1	1.6	75.7	1.2	2.1	54.7	1.3	2.6	40.9	1.5	3.1	31.5	1.6	3.7	24.3	1.8	4.3	18.7	2.1	4.9	15.2	2.3	5.4	12.8	2.5	5.8
130	111.0	1.1	1.6	78.7	1.2	2.1	56.9	1.3	2.6	42.5	1.5	3.1	32.7	1.6	3.7	25.3	1.8	4.3	19.4	2.1	4.9	15.9	2.3	5.4	13.4	2.5	5.8
135	115.3	1.1	1.6	81.7	1.2	2.1	59.1	1.3	2.6	44.2	1.5	3.1	34.0	1.6	3.7	26.3	1.8	4.3	20.2	2.0	4.9	16.6	2.3	5.4	14.1	2.5	5.8
140	119.6	1.1	1.6	84.7	1.2	2.1	61.3	1.3	2.6	45.8	1.5	3.1	35.2	1.6	3.7	27.3	1.8	4.3	21.0	2.0	4.9	17.2	2.3	5.4	14.7	2.5	5.8
145	123.8	1.1	1.6	87.8	1.2	2.1	63.5	1.3	2.6	47.5	1.5	3.1	36.5	1.6	3.7	28.7	1.8	4.3	21.8	2.0	4.9	17.9	2.3	5.4	15.3	2.5	5.8
150	128.1	1.1	1.6	90.8	1.2	2.1	65.7	1.3	2.6	49.1	1.5	3.1	37.8	1.6	3.7	29.7	1.8	4.3	22.6	2.0	4.9	18.6	2.3	5.4	15.9	2.4	5.8

RETARDANCE "D" AND "C"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-12 Parabolic Diversion Design Chart (Retardance “D” and “C”, Grade 2.00%)



V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "C"

Grade 2.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	5.9	0.9	1.5																								
10	12.4	0.8	1.5	8.1	0.9	2.0	5.9	1.0	2.5																		
15	18.5	0.8	1.5	12.3	0.9	2.0	9.3	1.0	2.5	6.8	1.1	3.0	4.7	1.4	3.5												
20	24.7	0.8	1.5	16.7	0.9	2.0	12.5	1.0	2.5	9.4	1.1	3.0	7.0	1.2	3.6	4.7	1.5	4.1									
25	30.8	0.8	1.5	20.8	0.9	2.0	15.9	1.0	2.4	11.8	1.1	3.0	9.0	1.2	3.5	6.8	1.3	4.1									
30	37.0	0.8	1.5	25.0	0.9	2.0	19.0	1.0	2.5	14.3	1.1	3.0	11.0	1.2	3.5	8.5	1.3	4.1	6.4	1.5	4.7						
35	43.2	0.8	1.5	29.1	0.9	2.0	22.2	1.0	2.5	16.9	1.0	3.0	12.9	1.1	3.5	10.1	1.3	4.1	7.8	1.4	4.7						
40	49.3	0.8	1.5	33.3	0.9	2.0	25.3	1.0	2.5	19.3	1.0	3.0	14.8	1.1	3.5	11.6	1.3	4.1	9.1	1.4	4.7	7.1	1.6	5.2			
45	55.5	0.8	1.5	37.4	0.9	2.0	28.5	1.0	2.5	21.7	1.0	3.0	16.7	1.1	3.5	13.1	1.3	4.1	10.4	1.4	4.7	8.2	1.6	5.2			
50	61.7	0.8	1.5	41.6	0.9	2.0	31.7	1.0	2.5	24.1	1.0	3.0	18.8	1.1	3.5	14.7	1.2	4.1	11.7	1.4	4.7	9.3	1.5	5.3	7.1	1.8	5.8
55	67.8	0.8	1.5	45.7	0.9	2.0	34.8	1.0	2.5	26.5	1.0	3.0	20.7	1.1	3.5	16.2	1.2	4.1	12.9	1.4	4.7	10.4	1.5	5.3	8.2	1.7	5.8
60	74.0	0.8	1.5	49.9	0.9	2.0	38.0	1.0	2.5	28.9	1.0	3.0	22.6	1.1	3.5	17.7	1.2	4.1	14.1	1.4	4.7	11.4	1.5	5.3	9.2	1.7	5.8
65	80.2	0.8	1.5	54.0	0.9	2.0	41.1	1.0	2.5	31.4	1.0	3.0	24.5	1.1	3.5	19.5	1.2	4.1	15.4	1.3	4.7	12.4	1.5	5.3	10.1	1.7	5.8
70	86.3	0.8	1.5	58.2	0.9	2.0	44.3	1.0	2.5	33.8	1.0	3.0	26.3	1.1	3.5	21.0	1.2	4.1	16.6	1.3	4.7	13.5	1.5	5.3	11.0	1.6	5.8
75	92.5	0.8	1.5	62.3	0.9	2.0	47.5	1.0	2.5	36.2	1.0	3.0	28.2	1.1	3.5	22.4	1.2	4.1	17.8	1.3	4.7	14.5	1.5	5.3	11.8	1.6	5.8
80	98.7	0.8	1.5	66.5	0.9	2.0	50.6	1.0	2.5	38.6	1.0	3.0	30.1	1.1	3.5	23.9	1.2	4.1	19.0	1.3	4.7	15.5	1.5	5.3	12.7	1.6	5.8
85	104.8	0.8	1.5	70.6	0.9	2.0	53.8	1.0	2.5	41.0	1.0	3.0	32.0	1.1	3.5	25.4	1.2	4.1	20.3	1.3	4.7	16.5	1.5	5.3	13.6	1.6	5.8
90	111.0	0.8	1.5	74.8	0.9	2.0	57.0	1.0	2.5	43.4	1.0	3.0	33.8	1.1	3.5	26.9	1.2	4.1	21.8	1.3	4.6	17.5	1.5	5.3	14.4	1.6	5.8
95	117.2	0.8	1.5	78.9	0.9	2.0	60.1	1.0	2.5	45.8	1.0	3.0	35.7	1.1	3.5	28.4	1.2	4.1	23.0	1.3	4.6	18.6	1.5	5.3	15.3	1.6	5.8
100	123.3	0.8	1.5	83.1	0.9	2.0	63.3	1.0	2.5	48.2	1.0	3.0	37.6	1.1	3.5	29.9	1.2	4.1	24.2	1.3	4.6	19.6	1.5	5.3	16.2	1.6	5.8
105	129.5	0.8	1.5	87.3	0.9	2.0	66.4	1.0	2.5	50.6	1.0	3.0	39.5	1.1	3.5	31.4	1.2	4.1	25.4	1.3	4.6	20.6	1.5	5.3	17.0	1.6	5.8
110	135.7	0.8	1.5	91.4	0.9	2.0	69.6	1.0	2.5	53.0	1.0	3.0	41.3	1.1	3.5	32.9	1.2	4.1	26.6	1.3	4.7	21.6	1.4	5.3	17.9	1.6	5.8
115	141.8	0.8	1.5	95.6	0.9	2.0	72.8	1.0	2.5	55.4	1.0	3.0	43.2	1.1	3.5	34.4	1.2	4.1	27.9	1.3	4.7	22.6	1.4	5.3	18.7	1.6	5.8
120	148.0	0.8	1.5	99.7	0.9	2.0	75.9	1.0	2.5	57.9	1.0	3.0	45.1	1.1	3.5	35.9	1.2	4.1	29.1	1.3	4.7	23.9	1.4	5.2	19.5	1.6	5.8
125	154.1	0.8	1.5	103.9	0.9	2.0	79.1	1.0	2.5	60.3	1.0	3.0	47.0	1.1	3.5	37.4	1.2	4.1	30.3	1.3	4.7	24.8	1.4	5.2	20.4	1.6	5.8
130	160.3	0.8	1.5	108.0	0.9	2.0	82.3	1.0	2.5	62.7	1.0	3.0	48.8	1.1	3.5	38.9	1.2	4.1	31.5	1.3	4.7	25.8	1.4	5.3	21.2	1.6	5.8
135	166.5	0.8	1.5	112.2	0.9	2.0	85.4	1.0	2.5	65.1	1.0	3.0	50.7	1.1	3.5	40.3	1.2	4.1	32.7	1.3	4.7	26.8	1.4	5.3	22.1	1.6	5.8
140	172.6	0.8	1.5	116.3	0.9	2.0	88.6	1.0	2.5	67.5	1.0	3.0	52.6	1.1	3.5	41.8	1.2	4.1	33.9	1.3	4.7	27.8	1.4	5.3	22.9	1.6	5.8
145	178.8	0.8	1.5	120.5	0.9	2.0	91.8	1.0	2.5	69.9	1.0	3.0	54.5	1.1	3.5	43.3	1.2	4.1	35.1	1.3	4.7	28.8	1.4	5.3	23.7	1.6	5.8
150	185.0	0.8	1.5	124.6	0.9	2.0	94.9	1.0	2.5	72.3	1.0	3.0	56.4	1.1	3.5	44.8	1.2	4.1	36.3	1.3	4.7	29.8	1.4	5.3	24.6	1.6	5.8

RETARDANCE "D" AND "C"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-13 Parabolic Diversion Design Chart (Retardance “D” and “C”, Grade 4.00%)

V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "C"

Grade 4.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	8.5	0.6	1.4	5.9	0.7	1.8	4.1	0.8	2.3																		
10	17.2	0.6	1.4	12.1	0.7	1.8	8.8	0.7	2.3	6.7	0.8	2.8	5.2	0.9	3.3	3.8	1.0	3.9									
15	25.8	0.6	1.4	18.1	0.7	1.8	13.4	0.7	2.3	10.3	0.8	2.8	8.1	0.8	3.4	6.4	0.9	3.9	4.9	1.0	4.5						
20	34.4	0.6	1.4	24.2	0.7	1.8	17.8	0.7	2.3	13.9	0.8	2.8	10.9	0.8	3.4	8.7	0.9	3.9	6.9	1.0	4.5	5.5	1.1	5.0			
25	43.0	0.6	1.4	30.2	0.7	1.9	22.3	0.7	2.3	17.4	0.8	2.8	13.8	0.8	3.3	10.9	0.9	3.9	8.8	1.0	4.5	7.1	1.0	5.1	5.7	1.2	5.6
30	51.6	0.6	1.4	36.3	0.7	1.9	26.7	0.7	2.3	20.8	0.8	2.8	16.5	0.8	3.3	13.2	0.9	3.9	10.7	0.9	4.5	8.7	1.0	5.1	7.1	1.1	5.6
35	60.2	0.6	1.4	42.3	0.7	1.9	31.1	0.7	2.3	24.3	0.8	2.8	19.3	0.8	3.4	15.6	0.9	3.9	12.5	0.9	4.5	10.3	1.0	5.0	8.4	1.1	5.6
40	68.8	0.6	1.4	48.3	0.7	1.9	35.6	0.7	2.3	27.8	0.8	2.8	22.0	0.8	3.4	17.8	0.9	3.9	14.4	0.9	4.5	11.8	1.0	5.0	9.8	1.1	5.7
45	77.4	0.6	1.4	54.4	0.7	1.9	40.0	0.7	2.4	31.2	0.8	2.8	24.8	0.8	3.4	20.0	0.9	3.9	16.4	0.9	4.4	13.3	1.0	5.0	11.1	1.1	5.7
50	86.0	0.6	1.4	60.4	0.7	1.9	44.5	0.7	2.4	34.7	0.8	2.8	27.5	0.8	3.4	22.2	0.9	3.9	18.2	0.9	4.4	14.9	1.0	5.0	12.3	1.1	5.7
55	94.6	0.6	1.4	66.5	0.7	1.9	48.9	0.7	2.4	38.2	0.8	2.8	30.3	0.8	3.4	24.4	0.9	3.9	20.0	0.9	4.4	16.6	1.0	5.0	13.6	1.1	5.7
60	103.2	0.6	1.4	72.5	0.7	1.9	53.4	0.7	2.4	41.7	0.8	2.8	33.0	0.8	3.4	26.6	0.9	3.9	21.8	0.9	4.5	18.1	1.0	5.0	14.9	1.1	5.7
65	111.8	0.6	1.4	78.5	0.7	1.9	57.8	0.7	2.4	45.1	0.8	2.8	35.8	0.8	3.4	28.9	0.9	3.9	23.6	0.9	4.5	19.6	1.0	5.0	16.2	1.1	5.7
70	120.4	0.6	1.4	84.6	0.7	1.9	62.3	0.7	2.4	48.6	0.8	2.8	38.6	0.8	3.4	31.1	0.9	3.9	25.4	0.9	4.5	21.1	1.0	5.0	17.7	1.1	5.6
75	129.0	0.6	1.4	90.6	0.7	1.9	66.7	0.7	2.4	52.1	0.8	2.8	41.3	0.8	3.4	33.3	0.9	3.9	27.2	0.9	4.5	22.6	1.0	5.0	19.0	1.1	5.6
80	137.6	0.6	1.4	96.7	0.7	1.9	71.2	0.7	2.4	55.5	0.8	2.8	44.1	0.8	3.4	35.5	0.9	3.9	29.1	0.9	4.5	24.1	1.0	5.0	20.2	1.1	5.6
85	146.2	0.6	1.4	102.7	0.7	1.9	75.6	0.7	2.4	59.0	0.8	2.8	46.8	0.8	3.4	37.7	0.9	3.9	30.9	0.9	4.5	25.6	1.0	5.0	21.5	1.1	5.6
90	154.8	0.6	1.4	108.7	0.7	1.9	80.0	0.7	2.4	62.5	0.8	2.8	49.6	0.8	3.4	39.9	0.9	3.9	32.7	0.9	4.5	27.1	1.0	5.0	22.8	1.1	5.6
95	163.4	0.6	1.4	114.8	0.7	1.9	84.5	0.7	2.4	65.9	0.8	2.8	52.3	0.8	3.4	42.2	0.9	3.9	34.5	0.9	4.5	28.6	1.0	5.0	24.0	1.1	5.6
100	172.0	0.6	1.4	120.8	0.7	1.9	88.9	0.7	2.4	69.4	0.8	2.8	55.1	0.8	3.4	44.4	0.9	3.9	36.3	0.9	4.5	30.1	1.0	5.0	25.3	1.1	5.6
105	180.6	0.6	1.4	126.9	0.7	1.9	93.4	0.7	2.4	72.9	0.8	2.8	57.8	0.8	3.4	46.6	0.9	3.9	38.1	0.9	4.5	31.6	1.0	5.0	26.5	1.1	5.6
110	189.2	0.6	1.4	132.9	0.7	1.9	97.8	0.7	2.4	76.3	0.8	2.8	60.6	0.8	3.4	48.8	0.9	3.9	39.9	0.9	4.5	33.1	1.0	5.0	27.8	1.1	5.6
115	197.8	0.6	1.4	138.9	0.7	1.9	102.3	0.7	2.4	79.8	0.8	2.8	63.3	0.8	3.4	51.0	0.9	3.9	41.7	0.9	4.5	34.6	1.0	5.0	29.0	1.1	5.6
120	206.4	0.6	1.4	145.0	0.7	1.9	106.7	0.7	2.4	83.3	0.8	2.8	66.1	0.8	3.4	53.3	0.9	3.9	43.6	0.9	4.5	36.1	1.0	5.0	30.2	1.1	5.7
125	215.0	0.6	1.4	151.0	0.7	1.9	111.2	0.7	2.4	86.8	0.8	2.8	68.8	0.8	3.4	55.5	0.9	3.9	45.4	0.9	4.5	37.6	1.0	5.0	31.5	1.1	5.7
130	223.7	0.6	1.4	157.1	0.7	1.9	115.6	0.7	2.4	90.2	0.8	2.8	71.6	0.8	3.4	57.7	0.9	3.9	47.2	0.9	4.5	39.1	1.0	5.0	32.7	1.1	5.7
135	232.3	0.6	1.4	163.1	0.7	1.9	120.1	0.7	2.4	93.7	0.8	2.8	74.3	0.8	3.4	59.9	0.9	3.9	49.0	0.9	4.5	40.6	1.0	5.0	34.0	1.1	5.7
140	240.9	0.6	1.4	169.1	0.7	1.9	124.5	0.7	2.4	97.2	0.8	2.8	77.1	0.8	3.4	62.1	0.9	3.9	50.8	0.9	4.5	42.1	1.0	5.0	35.2	1.1	5.7
145	249.5	0.6	1.4	175.2	0.7	1.9	129.0	0.7	2.4	100.6	0.8	2.8	79.8	0.8	3.4	64.3	0.9	3.9	52.6	0.9	4.5	43.6	1.0	5.0	36.5	1.1	5.7
150	258.1	0.6	1.4	181.2	0.7	1.9	133.4	0.7	2.4	104.1	0.8	2.8	82.6	0.8	3.4	66.6	0.9	3.9	54.4	0.9	4.5	45.1	1.0	5.0	37.8	1.1	5.7

RETARDANCE "D" AND "C"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second;
Depth "D" does not include allowance for freeboard or settlement.

Table DV-14 Parabolic Diversion Design Chart (Retardance “D” and “C”, Grade 6.00%)

V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "C"

Grade 6.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	10.6	0.5	1.3	7.3	0.6	1.8	5.3	0.6	2.3	4.0	0.7	2.8	2.9	0.8	3.2												
10	21.1	0.5	1.3	14.7	0.6	1.8	10.9	0.6	2.3	8.4	0.7	2.8	6.6	0.7	3.2	5.3	0.8	3.8	4.2	0.8	4.3						
15	31.6	0.5	1.3	22.1	0.6	1.8	16.3	0.6	2.3	12.7	0.6	2.7	10.1	0.7	3.3	8.2	0.7	3.8	6.6	0.8	4.3	5.4	0.9	4.9	4.3	1.0	5.5
20	42.1	0.5	1.3	29.5	0.6	1.8	21.7	0.6	2.3	17.0	0.6	2.7	13.6	0.7	3.2	11.1	0.7	3.7	9.0	0.8	4.3	7.4	0.8	4.9	6.1	0.9	5.5
25	52.7	0.5	1.3	36.8	0.6	1.8	27.1	0.6	2.3	21.2	0.6	2.8	17.0	0.7	3.2	13.9	0.7	3.8	11.3	0.8	4.3	9.3	0.8	4.9	7.8	0.9	5.5
30	63.2	0.5	1.3	44.2	0.6	1.8	32.5	0.6	2.3	25.4	0.6	2.8	20.4	0.7	3.2	16.6	0.7	3.8	13.7	0.8	4.3	11.3	0.8	4.9	9.4	0.9	5.5
35	73.7	0.5	1.3	51.6	0.6	1.8	38.0	0.6	2.3	29.7	0.6	2.8	23.8	0.7	3.2	19.4	0.7	3.8	16.0	0.8	4.3	13.4	0.8	4.9	11.1	0.9	5.5
40	84.2	0.5	1.3	58.9	0.6	1.8	43.4	0.6	2.3	33.9	0.6	2.8	27.2	0.7	3.3	22.2	0.7	3.8	18.3	0.8	4.3	15.3	0.8	4.9	12.7	0.9	5.5
45	94.8	0.5	1.3	66.3	0.6	1.8	48.8	0.6	2.3	38.2	0.6	2.8	30.7	0.7	3.3	24.9	0.7	3.8	20.6	0.8	4.3	17.2	0.8	4.9	14.5	0.9	5.4
50	105.3	0.5	1.3	73.6	0.6	1.8	54.2	0.6	2.3	42.4	0.6	2.8	34.1	0.7	3.3	27.7	0.7	3.8	22.8	0.8	4.3	19.1	0.8	4.9	16.1	0.9	5.4
55	115.8	0.5	1.3	81.0	0.6	1.8	59.7	0.6	2.3	46.6	0.6	2.8	37.5	0.7	3.3	30.5	0.7	3.8	25.1	0.8	4.3	21.0	0.8	4.9	17.7	0.9	5.4
60	126.4	0.5	1.3	88.4	0.6	1.8	65.1	0.6	2.3	50.9	0.6	2.8	40.9	0.7	3.3	33.3	0.7	3.8	27.4	0.8	4.3	22.9	0.8	4.9	19.3	0.9	5.4
65	136.9	0.5	1.3	95.7	0.6	1.8	70.5	0.6	2.3	55.1	0.6	2.8	44.3	0.7	3.3	36.0	0.7	3.8	29.7	0.8	4.3	24.8	0.8	4.9	20.9	0.9	5.4
70	147.4	0.5	1.3	103.1	0.6	1.8	75.9	0.6	2.3	59.3	0.6	2.8	47.7	0.7	3.3	38.8	0.7	3.8	32.0	0.8	4.3	26.7	0.8	4.9	22.5	0.9	5.4
75	158.0	0.5	1.3	110.5	0.6	1.8	81.3	0.6	2.3	63.6	0.6	2.8	51.1	0.7	3.3	41.6	0.7	3.8	34.3	0.8	4.3	28.6	0.8	4.9	24.1	0.9	5.4
80	168.5	0.5	1.3	117.8	0.6	1.8	86.8	0.6	2.3	67.8	0.6	2.8	54.5	0.7	3.3	44.3	0.7	3.8	36.5	0.8	4.3	30.5	0.8	4.9	25.7	0.9	5.5
85	179.0	0.5	1.3	125.2	0.6	1.8	92.2	0.6	2.3	72.0	0.6	2.8	57.9	0.7	3.3	47.1	0.7	3.8	38.8	0.8	4.3	32.4	0.8	4.9	27.3	0.9	5.5
90	189.6	0.5	1.3	132.6	0.6	1.8	97.6	0.6	2.3	76.3	0.6	2.8	61.3	0.7	3.3	49.9	0.7	3.8	41.1	0.8	4.3	34.3	0.8	4.9	28.9	0.9	5.5
95	200.1	0.5	1.3	139.9	0.6	1.8	103.0	0.6	2.3	80.5	0.6	2.8	64.7	0.7	3.3	52.6	0.7	3.8	43.4	0.8	4.3	36.2	0.8	4.9	30.5	0.9	5.5
100	210.6	0.5	1.3	147.3	0.6	1.8	108.5	0.6	2.3	84.8	0.6	2.8	68.1	0.7	3.3	55.4	0.7	3.8	45.7	0.8	4.3	38.1	0.8	4.9	32.1	0.9	5.5
105	221.1	0.5	1.3	154.6	0.6	1.8	113.9	0.6	2.3	89.0	0.6	2.8	71.5	0.7	3.3	58.2	0.7	3.8	47.9	0.8	4.3	40.0	0.8	4.9	33.7	0.9	5.5
110	231.7	0.5	1.3	162.0	0.6	1.8	119.3	0.6	2.3	93.2	0.6	2.8	74.9	0.7	3.3	60.9	0.7	3.8	50.2	0.8	4.3	41.9	0.8	4.9	35.3	0.9	5.5
115	242.2	0.5	1.3	169.4	0.6	1.8	124.7	0.6	2.3	97.5	0.6	2.8	78.3	0.7	3.3	63.7	0.7	3.8	52.5	0.8	4.3	43.8	0.8	4.9	36.9	0.9	5.5
120	252.7	0.5	1.3	176.7	0.6	1.8	130.2	0.6	2.3	101.7	0.6	2.8	81.7	0.7	3.3	66.5	0.7	3.8	54.8	0.8	4.3	45.7	0.8	4.9	38.5	0.9	5.5
125	263.3	0.5	1.3	184.1	0.6	1.8	135.6	0.6	2.3	106.0	0.6	2.8	85.1	0.7	3.3	69.3	0.7	3.8	57.1	0.8	4.3	47.6	0.8	4.9	40.1	0.9	5.5
130	273.8	0.5	1.3	191.5	0.6	1.8	141.0	0.6	2.3	110.2	0.6	2.8	88.5	0.7	3.3	72.0	0.7	3.8	59.4	0.8	4.3	49.5	0.8	4.9	41.7	0.9	5.5
135	284.3	0.5	1.3	198.8	0.6	1.8	146.4	0.6	2.3	114.4	0.6	2.8	91.9	0.7	3.3	74.8	0.7	3.8	61.6	0.8	4.3	51.4	0.8	4.9	43.3	0.9	5.5
140	294.9	0.5	1.3	206.2	0.6	1.8	151.8	0.6	2.3	118.7	0.6	2.8	95.3	0.7	3.3	77.6	0.7	3.8	63.9	0.8	4.3	53.3	0.8	4.9	44.9	0.9	5.5
145	305.4	0.5	1.3	213.6	0.6	1.8	157.3	0.6	2.3	122.9	0.6	2.8	98.7	0.7	3.3	80.3	0.7	3.8	66.2	0.8	4.3	55.2	0.8	4.9	46.5	0.9	5.5
150	315.9	0.5	1.3	220.9	0.6	1.8	162.7	0.6	2.3	127.1	0.6	2.8	102.1	0.7	3.3	83.1	0.7	3.8	68.5	0.8	4.3	57.1	0.8	4.9	48.1	0.9	5.5

RETARDANCE "D" AND "C"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second:
Depth "D" does not include allowance for freeboard or settlement.

Table DV-15 Parabolic Diversion Design Chart (Retardance "D" and "C", Grade 8.00%)

V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH' (D), AND V2 FOR RETARDANCE "C"

Grade 8.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	12.0	0.5	1.3	8.5	0.5	1.7	6.2	0.5	2.2	4.6	0.6	2.7	3.7	0.6	3.2	2.9	0.7	3.6									
10	24.1	0.5	1.3	16.9	0.5	1.7	12.6	0.5	2.2	9.6	0.6	2.7	7.8	0.6	3.2	6.3	0.6	3.7	5.1	0.7	4.2	4.2	0.8	4.8	3.2	0.9	5.3
15	36.1	0.5	1.3	25.3	0.5	1.7	18.9	0.5	2.2	14.4	0.6	2.7	11.8	0.6	3.2	9.7	0.6	3.7	7.9	0.7	4.2	6.5	0.7	4.8	5.4	0.8	5.3
20	48.1	0.5	1.3	33.8	0.5	1.7	25.2	0.5	2.2	19.2	0.6	2.7	15.8	0.6	3.2	12.9	0.6	3.7	10.7	0.7	4.2	8.8	0.7	4.8	7.4	0.8	5.3
25	60.1	0.5	1.3	42.2	0.5	1.7	31.5	0.5	2.2	24.0	0.6	2.7	19.7	0.6	3.2	16.2	0.6	3.7	13.4	0.7	4.2	11.2	0.7	4.7	9.3	0.8	5.3
30	72.1	0.5	1.3	50.6	0.5	1.7	37.8	0.5	2.2	28.8	0.6	2.7	23.6	0.6	3.2	19.4	0.6	3.7	16.1	0.7	4.2	13.5	0.7	4.8	11.3	0.7	5.3
35	84.1	0.5	1.3	59.1	0.5	1.7	44.1	0.5	2.2	33.6	0.6	2.7	27.6	0.6	3.2	22.6	0.6	3.7	18.7	0.7	4.2	15.7	0.7	4.8	13.3	0.7	5.3
40	96.2	0.5	1.3	67.5	0.5	1.7	50.4	0.5	2.2	38.4	0.6	2.7	31.5	0.6	3.2	25.8	0.6	3.7	21.4	0.7	4.2	17.9	0.7	4.8	15.2	0.7	5.3
45	108.2	0.5	1.3	76.0	0.5	1.7	56.7	0.5	2.2	43.2	0.6	2.7	35.4	0.6	3.2	29.0	0.6	3.7	24.1	0.7	4.2	20.2	0.7	4.8	17.1	0.7	5.3
50	120.2	0.5	1.3	84.4	0.5	1.7	63.0	0.5	2.2	48.0	0.6	2.7	39.4	0.6	3.2	32.3	0.6	3.7	26.8	0.7	4.2	22.4	0.7	4.8	19.0	0.7	5.3
55	132.2	0.5	1.3	92.8	0.5	1.7	69.3	0.5	2.2	52.8	0.6	2.7	43.3	0.6	3.2	35.5	0.6	3.7	29.4	0.7	4.2	24.7	0.7	4.8	20.9	0.7	5.3
60	144.2	0.5	1.3	101.3	0.5	1.7	75.6	0.5	2.2	57.6	0.6	2.7	47.2	0.6	3.2	38.7	0.6	3.7	32.1	0.7	4.2	26.9	0.7	4.8	22.8	0.7	5.3
65	156.3	0.5	1.3	109.7	0.5	1.7	81.8	0.5	2.2	62.4	0.6	2.7	51.2	0.6	3.2	41.9	0.6	3.7	34.8	0.7	4.2	29.1	0.7	4.8	24.7	0.7	5.3
70	168.3	0.5	1.3	118.2	0.5	1.7	88.1	0.5	2.2	67.2	0.6	2.7	55.1	0.6	3.2	45.2	0.6	3.7	37.5	0.7	4.2	31.4	0.7	4.8	26.6	0.7	5.3
75	180.3	0.5	1.3	126.6	0.5	1.7	94.4	0.5	2.2	72.0	0.6	2.7	59.0	0.6	3.2	48.4	0.6	3.7	40.1	0.7	4.2	33.6	0.7	4.8	28.5	0.7	5.3
80	192.3	0.5	1.3	135.0	0.5	1.7	100.7	0.5	2.2	76.8	0.6	2.7	63.0	0.6	3.2	51.6	0.6	3.7	42.8	0.7	4.2	35.9	0.7	4.8	30.3	0.7	5.3
85	204.3	0.5	1.3	143.5	0.5	1.7	107.0	0.5	2.2	81.6	0.6	2.7	66.9	0.6	3.2	54.9	0.6	3.7	45.5	0.7	4.2	38.1	0.7	4.8	32.2	0.7	5.3
90	216.4	0.5	1.3	151.9	0.5	1.7	113.3	0.5	2.2	86.4	0.6	2.7	70.8	0.6	3.2	58.1	0.6	3.7	48.1	0.7	4.2	40.3	0.7	4.8	34.1	0.7	5.3
95	228.4	0.5	1.3	160.3	0.5	1.7	119.6	0.5	2.2	91.2	0.6	2.7	74.8	0.6	3.2	61.3	0.6	3.7	50.8	0.7	4.2	42.6	0.7	4.8	36.0	0.7	5.3
100	240.4	0.5	1.3	168.8	0.5	1.7	125.9	0.5	2.2	96.0	0.6	2.7	78.7	0.6	3.2	64.5	0.6	3.7	53.5	0.7	4.2	44.8	0.7	4.8	37.9	0.7	5.3
105	252.4	0.5	1.3	177.2	0.5	1.7	132.2	0.5	2.2	100.8	0.6	2.7	82.6	0.6	3.2	67.8	0.6	3.7	56.2	0.7	4.2	47.1	0.7	4.8	39.8	0.7	5.3
110	264.4	0.5	1.3	185.7	0.5	1.7	138.5	0.5	2.2	105.6	0.6	2.7	86.6	0.6	3.2	71.0	0.6	3.7	58.8	0.7	4.2	49.3	0.7	4.8	41.7	0.7	5.3
115	276.5	0.5	1.3	194.1	0.5	1.7	144.8	0.5	2.2	110.4	0.6	2.7	90.5	0.6	3.2	74.2	0.6	3.7	61.5	0.7	4.2	51.5	0.7	4.8	43.6	0.7	5.3
120	288.5	0.5	1.3	202.5	0.5	1.7	151.1	0.5	2.2	115.2	0.6	2.7	94.4	0.6	3.2	77.4	0.6	3.7	64.2	0.7	4.2	53.8	0.7	4.8	45.5	0.7	5.3
125	300.5	0.5	1.3	211.0	0.5	1.7	157.4	0.5	2.2	120.0	0.6	2.7	98.4	0.6	3.2	80.7	0.6	3.7	66.9	0.7	4.2	56.0	0.7	4.8	47.4	0.7	5.3
130	312.5	0.5	1.3	219.4	0.5	1.7	163.7	0.5	2.2	124.8	0.6	2.7	102.3	0.6	3.2	83.9	0.6	3.7	69.5	0.7	4.2	58.3	0.7	4.8	49.3	0.7	5.3
135	324.5	0.5	1.3	227.9	0.5	1.7	170.0	0.5	2.2	129.6	0.6	2.7	106.2	0.6	3.2	87.1	0.6	3.7	72.2	0.7	4.2	60.5	0.7	4.8	51.2	0.7	5.3
140	336.6	0.5	1.3	236.3	0.5	1.7	176.3	0.5	2.2	134.4	0.6	2.7	110.2	0.6	3.2	90.3	0.6	3.7	74.9	0.7	4.2	62.7	0.7	4.8	53.1	0.7	5.3
145	348.6	0.5	1.3	244.7	0.5	1.7	182.6	0.5	2.2	139.2	0.6	2.7	114.1	0.6	3.2	93.6	0.6	3.7	77.6	0.7	4.2	65.0	0.7	4.8	55.0	0.7	5.3
150	360.6	0.5	1.3	253.2	0.5	1.7	188.9	0.5	2.2	144.0	0.6	2.7	118.0	0.6	3.2	96.8	0.6	3.7	80.2	0.7	4.2	67.2	0.7	4.8	56.9	0.7	5.3

RETARDANCE "D" AND "C"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second
Depth "D" does not include allowance for freeboard or settlement

Table DV-16 Parabolic Diversion Design Chart (Retardance “D” and “C”, Grade 10.00%)

V1 FOR RETARDANCE "D", TOP WIDTH (T), DEPTH (D), AND V2 FOR RETARDANCE "C"

Grade 10.00 Percent

Q CFS	V1=2.0			V1=2.5			V1=3.0			V1=3.5			V1=4.0			V1=4.5			V1=5.0			V1=5.5			V1=6.0		
	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2	T	D	V2
5	13.3	0.4	1.3	9.4	0.5	1.7	6.8	0.5	2.2	5.3	0.5	2.6	4.1	0.6	3.2	3.4	0.6	3.6	2.6	0.7	4.1						
10	26.6	0.4	1.3	18.7	0.5	1.7	13.8	0.5	2.2	10.9	0.5	2.6	8.5	0.6	3.2	7.1	0.6	3.6	5.9	0.6	4.1	4.9	0.7	4.7	4.0	0.7	5.3
15	39.9	0.4	1.3	28.0	0.5	1.7	20.7	0.5	2.2	16.3	0.5	2.6	12.8	0.6	3.2	10.9	0.6	3.6	9.0	0.6	4.1	7.5	0.6	4.7	6.3	0.7	5.2
20	53.2	0.4	1.3	37.4	0.5	1.7	27.6	0.5	2.2	21.7	0.5	2.7	17.0	0.6	3.2	14.5	0.6	3.6	12.1	0.6	4.1	10.2	0.6	4.6	8.5	0.7	5.2
25	66.5	0.4	1.3	46.7	0.5	1.7	34.5	0.5	2.2	27.1	0.5	2.7	21.3	0.6	3.2	18.1	0.6	3.6	15.1	0.6	4.1	12.7	0.6	4.7	10.8	0.7	5.2
30	79.8	0.4	1.3	56.1	0.5	1.7	41.4	0.5	2.2	32.5	0.5	2.7	25.5	0.6	3.2	21.7	0.6	3.6	18.1	0.6	4.1	15.2	0.6	4.7	12.9	0.7	5.2
35	93.1	0.4	1.3	65.4	0.5	1.7	48.3	0.5	2.2	37.9	0.5	2.7	29.8	0.6	3.2	25.3	0.6	3.6	21.1	0.6	4.1	17.8	0.6	4.7	15.1	0.7	5.2
40	106.4	0.4	1.3	74.7	0.5	1.7	55.2	0.5	2.2	43.3	0.5	2.7	34.0	0.6	3.2	29.0	0.6	3.6	24.1	0.6	4.1	20.3	0.6	4.7	17.2	0.7	5.2
45	119.7	0.4	1.3	84.1	0.5	1.7	62.1	0.5	2.2	48.8	0.5	2.7	38.3	0.6	3.2	32.6	0.6	3.6	27.2	0.6	4.1	22.8	0.6	4.7	19.4	0.7	5.2
50	133.0	0.4	1.3	93.4	0.5	1.7	69.0	0.5	2.2	54.2	0.5	2.7	42.5	0.6	3.2	36.2	0.6	3.6	30.2	0.6	4.1	25.4	0.6	4.7	21.5	0.7	5.2
55	146.3	0.4	1.3	102.8	0.5	1.7	75.9	0.5	2.2	59.6	0.5	2.7	46.8	0.6	3.2	39.8	0.6	3.6	33.2	0.6	4.1	27.9	0.6	4.7	23.7	0.7	5.2
60	159.6	0.4	1.3	112.1	0.5	1.7	82.8	0.5	2.2	65.0	0.5	2.7	51.0	0.6	3.2	43.4	0.6	3.6	36.2	0.6	4.1	30.5	0.6	4.7	25.9	0.7	5.2
65	172.9	0.4	1.3	121.4	0.5	1.7	89.7	0.5	2.2	70.4	0.5	2.7	55.3	0.6	3.2	47.1	0.6	3.6	39.2	0.6	4.1	33.0	0.6	4.7	28.0	0.7	5.2
70	186.2	0.4	1.3	130.8	0.5	1.7	96.6	0.5	2.2	75.8	0.5	2.7	59.5	0.6	3.2	50.7	0.6	3.6	42.2	0.6	4.1	35.5	0.6	4.7	30.2	0.7	5.2
75	199.5	0.4	1.3	140.1	0.5	1.7	103.5	0.5	2.2	81.2	0.5	2.7	63.8	0.6	3.2	54.3	0.6	3.6	45.2	0.6	4.1	38.1	0.6	4.7	32.3	0.7	5.2
80	212.8	0.4	1.3	149.5	0.5	1.7	110.5	0.5	2.2	86.7	0.5	2.7	68.0	0.6	3.2	57.9	0.6	3.6	48.3	0.6	4.1	40.6	0.6	4.7	34.5	0.7	5.2
85	226.1	0.4	1.3	158.8	0.5	1.7	117.4	0.5	2.2	92.1	0.5	2.7	72.3	0.6	3.2	61.5	0.6	3.6	51.3	0.6	4.1	43.1	0.6	4.7	36.6	0.7	5.2
90	239.4	0.4	1.3	168.1	0.5	1.7	124.3	0.5	2.2	97.5	0.5	2.7	76.5	0.6	3.2	65.2	0.6	3.6	54.3	0.6	4.1	45.7	0.6	4.7	38.8	0.7	5.2
95	252.7	0.4	1.3	177.5	0.5	1.7	131.2	0.5	2.2	102.9	0.5	2.7	80.8	0.6	3.2	68.8	0.6	3.6	57.3	0.6	4.1	48.2	0.6	4.7	40.9	0.7	5.2
100	266.0	0.4	1.3	186.8	0.5	1.7	138.1	0.5	2.2	108.3	0.5	2.7	85.0	0.6	3.2	72.4	0.6	3.6	60.3	0.6	4.1	50.7	0.6	4.7	43.1	0.7	5.2
105	279.3	0.4	1.3	196.2	0.5	1.7	145.0	0.5	2.2	113.7	0.5	2.7	89.3	0.6	3.2	76.0	0.6	3.6	63.3	0.6	4.1	53.3	0.6	4.7	45.2	0.7	5.2
110	292.6	0.4	1.3	205.5	0.5	1.7	151.9	0.5	2.2	119.2	0.5	2.7	93.5	0.6	3.2	79.6	0.6	3.6	66.4	0.6	4.1	55.8	0.6	4.7	47.4	0.7	5.2
115	305.9	0.4	1.3	214.9	0.5	1.7	158.8	0.5	2.2	124.6	0.5	2.7	97.8	0.6	3.2	83.3	0.6	3.6	69.4	0.6	4.1	58.3	0.6	4.7	49.5	0.7	5.3
120	319.2	0.4	1.3	224.2	0.5	1.7	165.7	0.5	2.2	130.0	0.5	2.7	102.0	0.6	3.2	86.9	0.6	3.6	72.4	0.6	4.1	60.9	0.6	4.7	51.7	0.7	5.3
125	332.5	0.4	1.3	233.5	0.5	1.7	172.6	0.5	2.2	135.4	0.5	2.7	106.3	0.6	3.2	90.5	0.6	3.6	75.4	0.6	4.1	63.4	0.6	4.7	53.8	0.7	5.3
130	345.8	0.4	1.3	242.9	0.5	1.7	179.5	0.5	2.2	140.8	0.5	2.7	110.5	0.6	3.2	94.1	0.6	3.6	78.4	0.6	4.1	66.0	0.6	4.7	56.0	0.7	5.3
135	359.1	0.4	1.3	252.2	0.5	1.7	186.4	0.5	2.2	146.2	0.5	2.7	114.8	0.6	3.2	97.7	0.6	3.6	81.4	0.6	4.1	68.5	0.6	4.7	58.1	0.7	5.3
140	372.4	0.4	1.3	261.6	0.5	1.7	193.3	0.5	2.2	151.7	0.5	2.7	119.0	0.6	3.2	101.3	0.6	3.6	84.4	0.6	4.1	71.0	0.6	4.7	60.3	0.7	5.3
145	385.7	0.4	1.3	270.9	0.5	1.7	200.2	0.5	2.2	157.1	0.5	2.7	123.3	0.6	3.2	105.0	0.6	3.6	87.5	0.6	4.1	73.6	0.6	4.7	62.5	0.7	5.3
150	399.0	0.4	1.3	280.2	0.5	1.7	207.1	0.5	2.2	162.5	0.5	2.7	127.5	0.6	3.2	108.6	0.6	3.6	90.5	0.6	4.1	76.1	0.6	4.7	64.6	0.7	5.3

RETARDANCE "D" AND "C"

NOTE: Width and Depth dimensions are in feet; Velocity measurements are in feet per second; Depth "D" does not include allowance for freeboard or settlement.

H. Sediment Barrier (SB)

Sediment Barrier (SB)

Practice Description

A sediment barrier is a temporary structure used across a landscape mostly on the contour to reduce the quantity of sediment that is moving downslope. The most commonly used barrier is a silt fence (a geotextile fabric that is trenched into the ground and attached to supporting posts and possibly reinforced with a wire fence or polypropylene netting). Other barrier materials could include sand bags, wattles, and various man-made materials and devices that can be used in a similar manner as a silt fence.

This practice applies where sheet and rill erosion occurs on small disturbed areas. Barriers intercept runoff from upslope to form ponds that temporarily store runoff and allow sediment to settle out of the water and remain on the construction site.

Planning Considerations

Sediment barriers may be used on developing sites. It is important that they be installed on the contour so that flow will not concentrate and cause overtopping due to lack of storage capacity. It is also important that the ends of sediment barriers are turned upslope to prevent runoff from bypass around the ends of the barrier. Prevention of scouring, erosion, and undermining at and under sediment barriers is also of upmost importance to ensure maximum impoundment capabilities.

The most commonly used sediment barriers are silt fences and manufactured sediment logs (often referred to as wattles or sediment retention fiber rolls). Manufactured sediment logs should be installed according to manufacturer's recommendations.

The success of silt fences depends on a proper installation (on the contour with each end turned up slope) that causes the fence to develop maximum efficiency of sediment trapping. Silt fences should be carefully installed to meet the intended purpose. Silt fences are effective at trapping coarse sediment but do not effectively reduce turbidity as water passes through the geotextile fabric.

A silt fence is specifically designed to retain sediment transported by sheet flow from disturbed areas, while allowing water to pass through the fence. Water flow through the silt fence often decreases over time as silts and trash "blind" or seal the geotextile fabric. Silt fences should be installed to be stable under the flows expected from the site. Generally, silt fences should not be installed across streams, ditches, waterways, or other concentrated flow areas. When properly designed and installed, silt fence can be used as a Check Dam (See Check Dam).

Silt fences are composed of geotextile (i.e., woven and non-woven) supported between steel or wooden posts. Silt fences are commercially available with geotextile attached to the post and can be rolled out and installed by driving the post into the ground. This type of silt fence is simple to install, but more expensive than some other installations. Silt fences must be trenched in at the bottom to prevent runoff from undermining the fence and developing rills under the fence. Locations with high runoff flows or velocities should use either a wire or polypropylene net reinforcement. In addition, decreasing the spacing between support posts will improve the structural integrity of the silt fence in these areas.

Design professionals should consider specifying an "off-set" trench installation. This involves a conventional 6 in. x 6 in. trench to bury the geotextile with the posts and wire installed 6 in. downslope of the trench. The wire is on top of the ground surface and not in a trench. This installation has proven to have less potential for undermining than any installation tested at the Auburn University Erosion and Sediment Control Test Facility.

A rather recent innovation that is still being tested and refined is referred to as a "sediment retention barrier with flocculant." It is used to introduce flocculant to turbid runoff causing flocculation. A sediment retention barrier should only be used in conjunction with effective erosion and sediment control practices upstream that have removed sediment and turbidity as much as possible without chemical additive. The measure consists of a double row of netting on the contour that allows runoff to easily pass through. Material such as jute is secured to the ground between the rows of netting and adjacent to the downslope row. Loose straw is placed between the rows (see Figure SB-1). An approved flocculant powder is added at a designed rate to all the jute and in layers within the straw. The measure is located upstream of sediment control (sediment basin, sediment trap, or sediment barrier) which will pond, allow for flocs to settle, and capture flocs prior to runoff leaving the site. Design professionals should get details needed to design this measure from a research professional or a qualified industry representative.

Figure SB-1 Sediment Retention Barrier

Design Criteria (for silt fence)

Silt fence installations are normally limited to situations in which only sheet or overland flow is expected because the practice cannot pass the volumes of water generated by channelized flows. Silt fences are normally constructed of synthetic fabric (geotextile) and the life is expected to be the duration of most construction projects. Silt fence geotextile should conform to the property requirements found in AASHTO M288 shown in Table SB-1 as follows:

Table SB-1 Silt Fence Geotextile Fabric Requirements per AASHTO M288

Requirement	Test Methods	Units	Type A supported fence	Type B unsupported fence
Grab Strength	ASTM			
Machine Direction	D4632/D4632M	N	400	550
X-Machine Direction			400	450
Permittivity	ASTM D4491	sec^{-1}	0.05	0.05
Apparent Opening Size	ASTM D4751	mm	0.60 max avg roll value	0.60 max avg roll value
Ultraviolet stability (retained strength)	ASTM D4355/4355M	%	70% after 500 h of exposure	70% after 500 h of exposure

Note: ALDOT has an approved products list for geotextile

The drainage area behind the silt fence should not exceed ¼ acre per 100 linear feet of silt fence for non-reinforced fence and ½ acre per 100 feet of reinforced silt fence. When all runoff from the drainage area is to be stored behind the fence (i.e. there is no stormwater disposal system in place) the maximum slope length behind the fence should not exceed those shown in Table SB-2.

Table SB-2 Slope Limitations for Silt Fence

Land Slope (Percent)	Maximum Slope Length Above Fence (Feet)
<2	100
2 to 5	75
5 to 10	50
10 to 20*	25
>20	15

*In areas where the slope is greater than 10%, a flat area length of 10 feet between the toe of the slope to the fence should be provided.

Type A Silt Fence

Type A fence shall be a minimum of 24" and not more than 32" above ground with wire reinforcements and is used on sites needing the highest degree of protection by a silt fence. The wire reinforcement is necessary because this type of silt fence is used for the highest flow situations and has almost 3 times the flow rate as Type B silt fence. Wire fence should be made of 14-gauge wire with 6 in. x 6 in. openings (Note: ALDOT wire spacing may differ). Type A silt fence should be used where runoff flows or velocities are particularly high or where slopes exceed a vertical height of 10 feet. Staked tie backs on each end of a Type A silt fence may be necessary to prevent overturning. Tie backs should also be used at points of possible concentration and overtopping if site conditions do not allow for the silt fence to be installed on the contour.

Provide a riprap splash pad with a geotextile underlay or other outlet protection device for any point where flow may overtop the silt fence.

The silt fence should be installed as shown in Figure SB-2. Maximum post spacing is 10 ft. In situations where runoff flows parallel with the silt fence when in perimeter control applications, 10 ft. spacing is adequate. J-hooks should also be considered for long parallel flow scenarios to slow flow velocity and create areas of impoundments, thereby reducing scour potential under the silt fence. For the portion of the silt fence that creates the J-hook impoundment area, the post spacing should be reduced to 5 ft. to support the hydrostatic loads. For all installations that intercept flow perpendicularly to the slope causing a concentrated impoundment, the maximum post spacing should be reduced to 5 ft. Materials for posts, post size, and fasteners are shown in Tables SB-3 and SB-4. Do not use "light weight" steel posts commonly found at building supply stores. Details for overlap of Type A silt fence is available from The Alabama Department of Transportation construction drawings.

Geotextile silt fence material should be looped over each post and the top of the wire to prevent sagging. A "hog ring" attachment should be made each 2 feet along the top of the wire.

Table SB-3 Post Size for Silt Fence

	Minimum Length	Type of Post	Size of Post
Type A	5'	Steel "T" Post	1.25 lb./ft. min.
Type B	4'	Soft Wood Oak Steel	3" diameter or 2X4 1.5" X 1.5" 1.25 lb./ft. min.

Table SB-4 Wood Post Fasteners for Silt Fence

	Gauge	Crown	Legs	Staples/Post
Wire Staples	17 min.	¾" wide	½" long	5 min.
	Gauge	**Length**	**Button Heads**	**Nail/Post**
Nails	14 min.	1"	¾" long	4 min.

Type B Silt Fence

This 36" wide geotextile fabric should be used on developments where the life of the project is short (6 months or less) and there is less need for protection from a silt fence.

The silt fence should be installed as shown in Figure SB-3. Post spacing is either 4 ft. or 6 ft. based on geotextile elongation % (see note on Figure SB-3). Materials for posts and fasteners are shown in Tables SB-3 and SB-4. Details for overlap of the silt fence and fastener placement are shown in Figure SB-4.

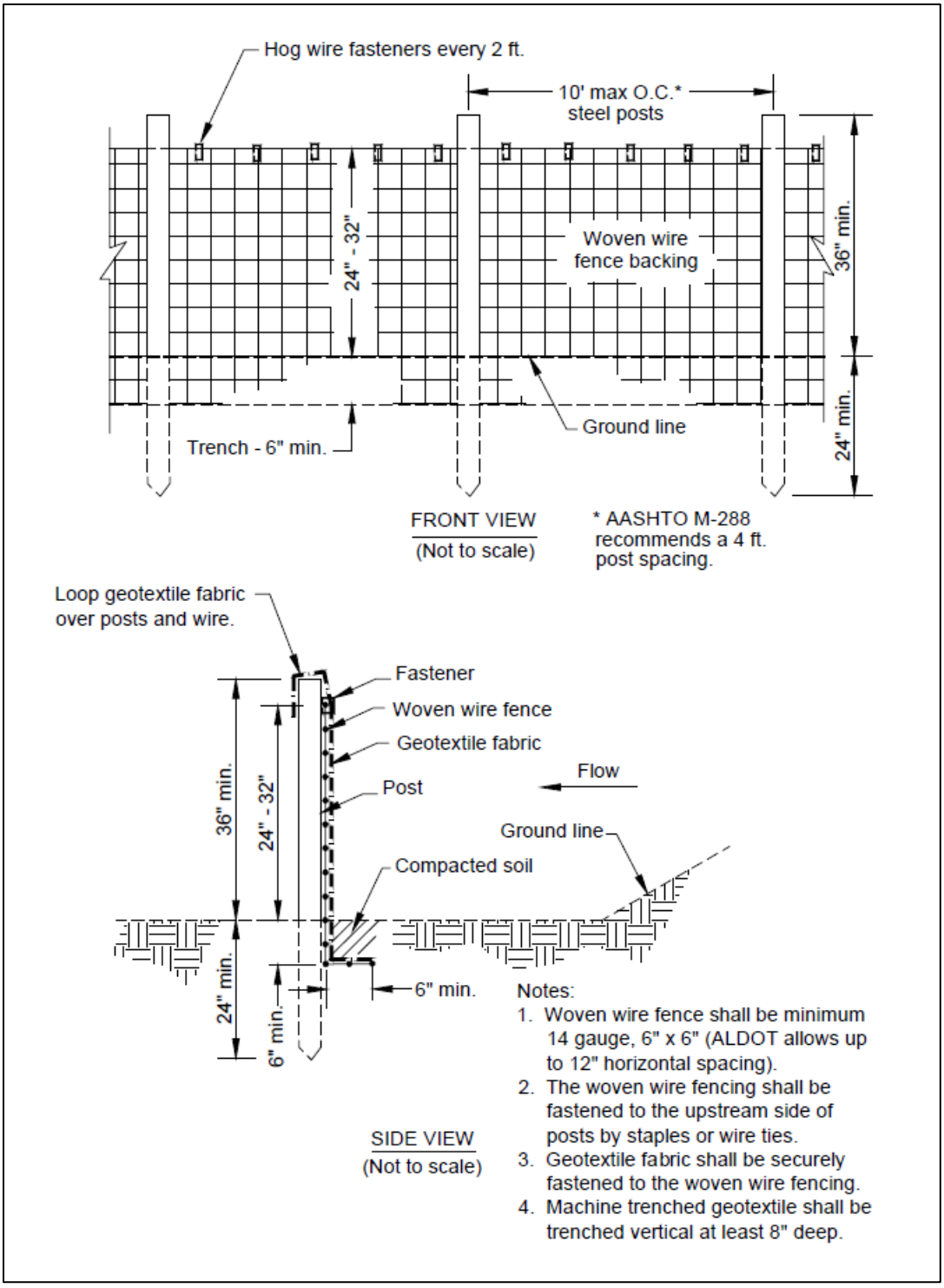


Figure SB-2 Silt Fence-Type A
(For post material requirements see Tables SB-3 and SB-4)

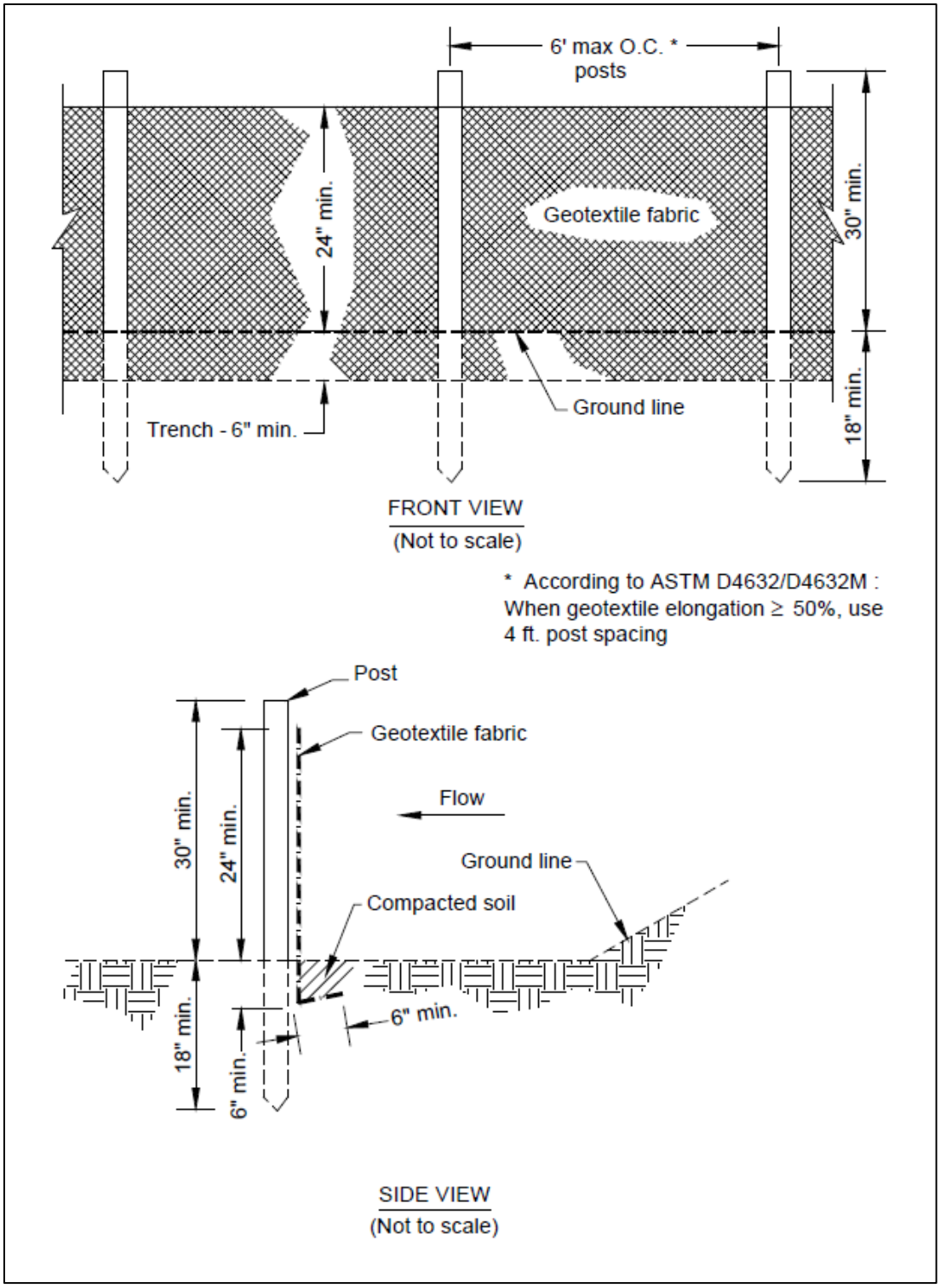


Figure SB-3 Silt Fence - Type B

(1) For post material requirements see Tables SB-3 and SB-4

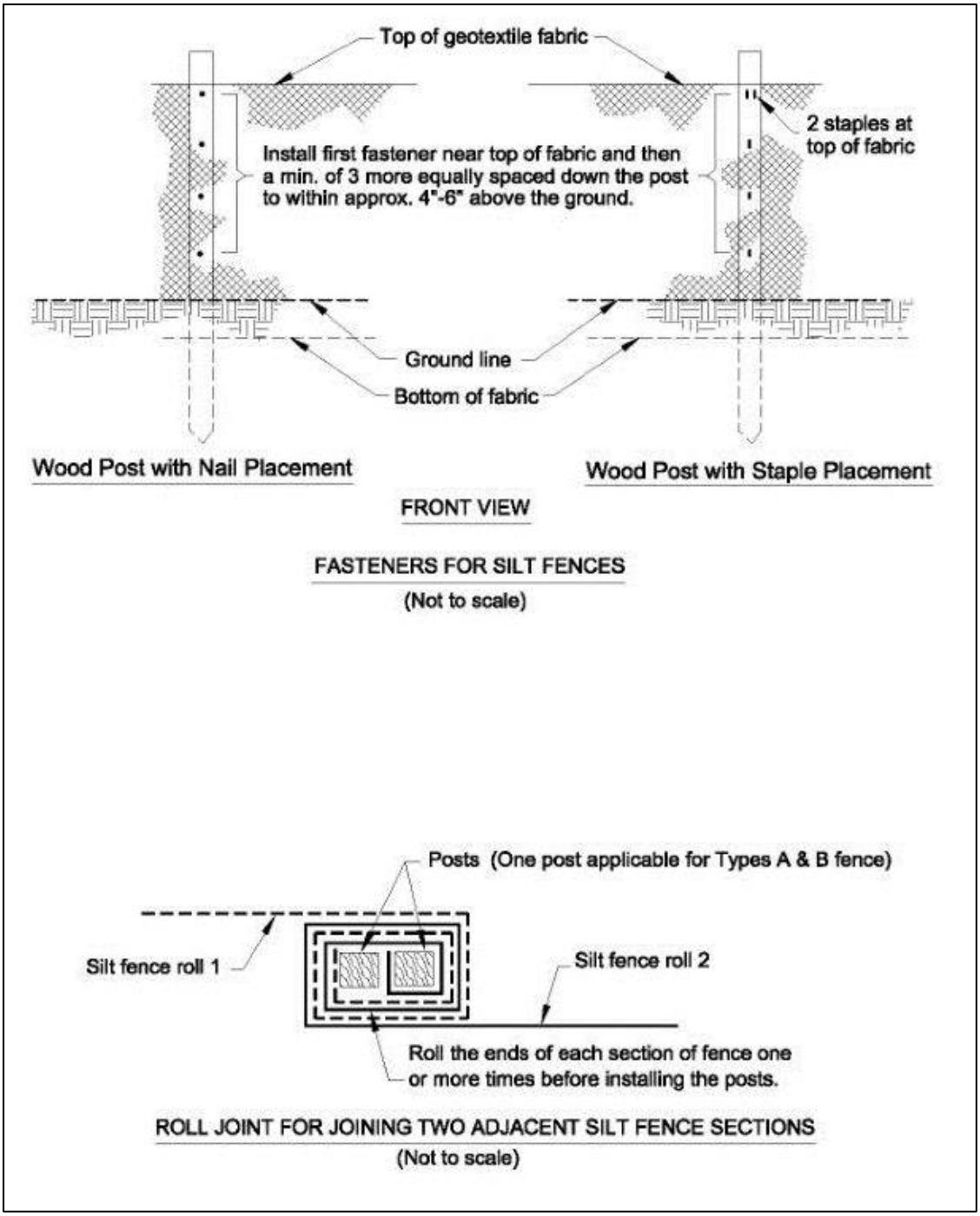


Figure SB-4 Silt Fence Installation Details

I. Corrective Action Log

Attachment I – Corrective Action Log

Project Name:
CBMPP Contact:

Inspection Date	Inspector Name(s)	Description of BMP Deficiency	Corrective Action Needed (including planned date/responsible person)	Date Action Taken/Responsible person

J. CBMPP Amendment Log

Attachment J - CBMPP Amendment Log

Project Name:
CBMPP Contact:

Amendment No.	Description of Amendment	Date of Amendment	Amendment Prepared by [Name(s) and Title]

K. Grading and Stabilization Log

Attachment K - Grading and Stabilization Activities Log

Project Name:
CBMPP Contact:

Date Grading Activity Initiated	Description of Grading Activity	Date Grading Activity Ceased (Indicate Temporary or Permanent)	Date When Stabilization Measures are Initiated	Description of Stabilization Measure(s) and Location(s)

L. Rainfall Documentation Log

Attachment L - Rainfall Documentation and Observations

Project Name or Description:
Project Location:
ADEM Permit Number:

Date	Start Time	End Time	Total Depth (to tenth of inch)	Observations (rainfall intense, moderate, etc.)	Initials

RESPONSES TO COMMENTS

Responses to Alabama Department of Environmental Management Comments on Revision 0 Corrective Measures Implementation (CMI) Work Plan, RSA-271, Former Boiler House, Building 7729, Operable Unit 10, dated January 13, 2021

Comments from Mr. Jason Wilson, Governmental Hazardous Waste Branch, Land Division:

Comment 1: **<u>Page 2-3, Section 2.3, Summary of RSA-271 Contaminant Fate and Transport Evaluation:</u> The section mentions that even though 1-methylnaphthalene, methylene chloride, and naphthalene were delineated and identified as chemicals of concern (COCs) for potential leaching from soil to groundwater in the RSA-271 RCRA Facility Investigation (RFI) Report, Redstone proposes No Further Action (NFA) for these COCs based on the information that a site-specific evaluation utilizing the ARBCA model software (ADEM, 2017b) for 1-methylnaphthane and naphthalene was completed and showed that neither of these poses a leaching threat to groundwater. However, the Department does not agree with the proposal of NFA for soils. It is unclear to the Department which ARBCA model was used. A synthetic precipitation leaching procedure (SPLP) analysis should be performed on the site soils to determine a site specific leaching potential or the complete list of COCs identified in the RFI should be addressed in the remedy.**

Response 1: This comment raises several questions, and each will be addressed separately.

1. **Methylene chloride**. See response to comment 3 with the revisions planned for clarity.

2. **It is unclear to the Department which ARBCA model was used.** Based on ADEM's comments, the values calculated by ADEM's software package are better understood if they are considered to be the RBTLs for soil. Therefore, the text of Section 2.3.3 will be moved to Section 3.4 where the results of ADEM's Groundwater Resource Protection Model, as per Sections 6.9 and 6.11 plus Appendix D of ADEM's 2017 ARBCA guidance will be cited. The text will also direct the reader more clearly to Appendix G. Note that this model is explained diagrammatically on Figure B-1 of ADEM's ARBCA guidance (ADEM, 2017).

3. **The Department does not agree with the proposal of NFA for soils.** Army agrees that SPLP testing is needed as part of the soil action for this site. Therefore, the Army proposes to collect four soil samples from two soil borings within the contaminated area and analyze these samples for their PAH content via SPLP (Attachment 1). The SPLP results will be evaluated per ADEM guidance as described in Section 4.2.1 "Direct SPLP Testing Of Soil Samples" of the RSA-271 RFI, (Aptim 2017) and with a consideration of the attenuation factor developed for transport from the source area to the selected point of exposure well, 271-RS2622, using the Domenico model as recommended in ADEM guidance (see Appendix E of

ADEM, 2017). Evaluation of the SPLP data will determine the need for a follow-on corrective measure for soil. Plans for a follow-on soil excavation will be included in the revised CMIP.

Comment 2: **<u>Page 2-3, Section 2.3.1, Leaching from Soil to Groundwater, 1-Methylnaphthalene and Naphthalene:</u> This section mentions that there were no subsurface soil samples that exceeded the Regional Screening Levels (RSLs) for COCs. However, with the exception of Figure G-1 in Appendix G, there were no graphical illustrations of the locations of the subsurface sample locations. Also, subsurface sample depths and analytical data that were collected during the RFI were not included in the document. Figures and analytical data associated with the RFI as well as the UST closure report should be provided to nullify the recommendations made in the RFI to address the COCs in soil as well as justify the recommendations made in the CMIP for NFA in soils.**

Response 2: Comment noted. Section 2.2.1 "Nature and Extent of Contamination, Soil" will be revised to contain separate discussions regarding surface soil (Section 2.2.1.1) and subsurface soil (Section 2.2.1.2). Figures and data included with the RFI will be summarized here for site soils. No UST closure report is available as the UST was unregulated.

Comment 3: **<u>Page 2-3, Section 2.3.1, Leaching from Soil to Groundwater, Sentence 5 and 6:</u> The sentences state "The RFI concluded that methylene chloride in soil had a potential, although slight, to threaten the underlying groundwater. However, further evaluation of the leaching potential for methylene chloride is warranted given several weight-of-evidence (WOE) factors." Please revise this section by clarifying the several WOE factors that warrants further evaluation of the leaching potential or reference the section in the CMI Work Plan that further discusses these factors.**

Response 3: This sentence will be restated as follows and the discussion of the lines of evidence presented in Section 2.3.3 will be moved to Section 2.3.1. The text will be revised as follows:

Methylene chloride. The RFI concluded that methylene chloride in soil had a potential, although slight, to pose a threat to the underlying groundwater from the soil to groundwater migration pathway. However, because there does not appear to be a soil source for methylene chloride, additional lines of evidence must be considered. These include:

- Methylene chloride is a common laboratory contaminant,
- It was detected at a concentration greater than the RSA-specific DAF4 SSL in only one subsurface soil sample out of a total of 34 subsurface soil samples (less than 3 percent).

- It was not detected at concentrations greater than the RSA-specific DAF4 SSL in surface soil.

- In the 36 groundwater samples collected for this site for VOCs, methylene chloride was not detected and thus does not exceed its RSL in any sample. These results include samples from groundwater monitoring well 271-RS1632, which is adjacent to 271-SB007. Given the operational history of this site, if a soil source were present, it would have migrated to groundwater by now which supports that no source is currently present.

- The sample from 271-SB007 (15 to 17 feet bgs), with the methylene chloride result that exceeds the DAF4 SSL was collected from soils directly below the level of the former tank bottom, and the VOC portion of the sample was flagged as requiring dilution by the laboratory due to the presence of petroleum fuel in the matrix. Common laboratory contaminants are often introduced during the sample cleanup and dilution process.

- The results from 271-RS1632 that was installed directly adjacent to 271-SB007) exhibited lower concentrations of VOCs and did not require dilution. This sample did not exhibit detected methylene chloride concentrations. Sample locations are presented on Figure 2-xx (figure to be developed per response to comment 2).

In conclusion, these results support that the single methylene chloride SSL exceedance is more likely the result of the analytical process rather than from site-related contamination. Therefore, action based on the soil to groundwater migration pathway for methylene chloride is not warranted since no soil source is present at the site.

Comment 4: **<u>Page 2-7, Section 2.4.2, Human Health ARBCA Evaluation, 3rd Paragraph:</u> This section mentions that a Preliminary Screening Level (PSL) evaluation involving Preliminary Screening Values (PSVs) was conducted for all chemicals that were determined to be site related. These concentrations of chemicals of concern (COCs) should be compared to the current Regional Screening Levels (RSLs) instead of PSVs in this CMIP and all future documents. Please address this discrepancy throughout the entire RSA-271 CMI Work Plan and revise accordingly.**

Response 4: Soil and groundwater data have been screened against the most recent RSLs (EPA, May 2021). No additional constituents in soil exceeded 2021 RSLs. In groundwater, two additional explosives, 2-Amino-4,6-dinitrotoluene and 4-Amino-2,6-dinitrotoluene exceeded 2021 RSLs. These two chemicals will be added to the groundwater monitoring program for this site as groundwater COCs. In addition, updated versions of Tables 3-6 through 3-8 from the RFI report will be added to Appendix G for ADEM's information.

Comment 5: **<u>Page 4-6, Section 4.2.6, Storm Water Erosion and Sediment Controls, Last Sentence:</u> The sentence states "Upon completion of the project, erosion and sediment control devices will be removed." Please revise this section by clarifying how the erosion and sedimentation control devices will be disposed or removed based on type of waste present or reference the section in the CMI Work Plan that further discusses the proper removal.**

Response 5: Erosion and sedimentation control devices, if employed at the site, will be removed from the ground, the loose dirt removed from the material and the silt fencing, etc. will be disposed of in a Subtitle D Landfill. This information will be added to the CMIP in Section 4.2.6. Note that if employed, erosion control and sedimentation devices will be placed around the perimeter of where the proposed monitoring well(s) will be drilled. There is no identified surface soil contamination outside the site boundary and therefore any requirement to dispose of the silt fencing, etc., as something other than construction debris is not warranted.

Comment 6: **<u>Page 4-8, Section 4.3, Groundwater Monitoring:</u> In this section there is a recommendation for four well locations to be included in the groundwater monitoring program for corrective measures. However, the Department does not agree with these recommended locations because they are inadequate to monitor for the COCs associated with the surface media site. The current well layout does not provide adequate monitoring locations to ensure that contaminant concentrations in the groundwater vicinity of monitoring wells 271-RS2707 and 271-RS2708 are actually biodegrading and not simply migrating away from the source area. Groundwater contour maps should be constructed from historical groundwater elevation data from the site to determine appropriate monitoring well placement locations.**

Response 6: Comment noted. However, consideration should be given to the fact that the local groundwater gradient, as mapped by the 3 existing wells at the RSA-271 site, may not provide an accurate depiction of smaller scale or semi-regional groundwater flow directions. For small surface media sites such as this sentinel well locations should be selected using the groundwater unit potentiometric surface map. The smaller scale map (i.e., groundwater unit scale) generally contains more well control which constrains the elevation contours and provides a more reproducible and accurate picture of groundwater flow across individual sites, in this case the RSA-271 site.

It is very probable that both biodegradation and dilution via transport by advective groundwater flow are contributing and will continue to contribute to plume attenuation. However, it is difficult to evaluate the impact of biodegradation as there has not been a sampling program designed with the intent to identify the attenuation mechanisms at work on the hydrocarbon plume beneath RSA-271.

The Army recommends installing and sampling wells at the downgradient, proposed locations illustrated on Figure 4-1 in the Revision 1 CMIP. Any future expansion of the Raytheon missile production facility may have a significant impact on the long-term existence of wells installed inside the perimeter security road/fence and therefore the Army requests that the agency concur with a monitoring program focused on sentinel or perimeter wells located either outside the perimeter security fence or adjacent to the perimeter security road. The proposed monitoring program would include the installation and sampling of three new wells and the sampling of three interior wells.

Responses to Alabama Department of Environmental Management Comments on Revision 1 Corrective Measures Implementation (CMI) Work Plan, RSA-271, Former Boiler House, Building 7729, Operable Unit 10, dated November 22, 2021

Comments from ADEM, Governmental Hazardous Waste Branch, February 24, 2022:

General Comment: **In response to the Department's previous comment that a synthetic precipitation leaching procedure (SPLP) should be performed on the site soils to determine a site-specific leaching potential, the Army proposes to collect four soil samples from two soil borings within the contaminated area and analyze these samples for the potential for leaching of polynuclear aromatic hydrocarbons (PAH) from soil to groundwater via SPLP. The Army should also collect groundwater samples at the top of the water table where the soil samples are being collected to assist in the determination if the contaminated soil may leach to the groundwater. Please address.**

Response to General Comment: Data collected from the water table will provide no additional information to support whether or not source material in soil is posing a current leaching threat to groundwater that requires an action. Groundwater results are the integrator of historical leaching from soil sources including tanks or pits. Based on the sample planning proposed, no additional information would be collected by sampling groundwater beneath the excavation. As shown on Figures 3-2 and 4-1, monitoring well 271-RS2707 is immediately adjacent to the planned excavation area and is the center of the plume core (see Figure 2-4). This well is less than 6 feet away from the planned excavation boundary. Baseline groundwater sampling will be performed prior to soil excavation (if needed) and will establish baseline conditions for groundwater. Monitoring for well 271-RS2707 during the MNA phase will provide information on the success of the corrective measures including, if warranted, soil removal and placement of biodegradation enhancements.

Lastly, ADEM has always emphasized that cleanup goals need to be clearly defined values. The cleanup goals for soil and SPLP leachate listed in the CMIP present represent clearly defined endpoints. Ongoing groundwater monitoring will provide both ADEM and Army information on if groundwater results are attenuating as planned and if not, additional remedies may be warranted in the future. No edits to the document appear to be warranted based on this comment.

Specific Comment 1: **This section mentions three proposed overburden monitoring wells for installation that will assist in the ability to gauge the effectiveness of the proposed remedy and better monitor downgradient groundwater**

concentrations. In a previous comment letter, the Department requested that current potentiometric surface maps from data used from the current onsite wells be provided to assist in determining adequate monitoring locations, however those were not provided. The Army's response to this request was that the three existing wells at RSA-271 may not provide an accurate depiction of smaller scale or semi-regional groundwater flow directions. Due to the different chemical characteristics and nature of petroleum products that were released at RSA-271, as compared to the solvents, explosives, and perchlorate that reportedly migrated onto RSA-271 from offsite sources, the Army should also install three shallow monitoring wells with screens intersecting the groundwater table that are nested beside the three proposed submerged wells to provide sufficient data that captures all of the COCs in the groundwater monitoring of this site. Based on groundwater flow directions after well installation, additional wells may be required to be installed.

Response 1: This comment has two portions and each will be addressed separately:

(Subcomment 1) *the Department requested that current potentiometric surface maps from data used from the current onsite wells be provided to assist in determining adequate monitoring locations, however those were not provided.*

Response to Subcomment 1 The Department fully supported the Brownfields Type development of the Raytheon Missile Facility. During this redevelopment, the majority of site-related as well as regional wells required closure as shown on Figure 2-3 in this CMIP. ADEM approved the RFI report without requiring well installation. As a result, it was not possible to provide current potentiometric surface maps based on the current onsite wells as the existing well layout of the site does not provide adequate coverage. This situation is being remedied in the CMI-C phase where three downgradient wells are planned for installation. Secondly, the Army has repeatedly demonstrated that semi-regional groundwater potentiometric surface maps are a more reliable indicator of groundwater flow than site-specific maps based on a handful of measurements. Many sites at RSA show localized potentiometric surface reversals that are inconsistent with semi-regional flow maps. Lastly, if groundwater flow direction during the MNA program does not appear to be consistent with the expected flow direction, additional wells can be installed in other locations to the north or northeast of the site. This contingency will be added to Chapter 5 of this CMIP report to acknowledge that groundwater flow direction may change over time and the monitoring well layout may need to be adjusted accordingly.

(Subcomment 2) Due to the different chemical characteristics and nature of petroleum products that were released at RSA-271, as compared to the solvents, explosives, and perchlorate that reportedly migrated onto RSA-271

from offsite sources, the Army should also install three shallow monitoring wells with screens intersecting the groundwater table that are nested beside the three proposed submerged wells to provide sufficient data that captures all of the COCs in the groundwater monitoring of this site.

Response to Subcomment 2

It is a common misunderstanding that constituents that are capable of forming DNAPLs or LNAPLs behave in a similar fashion to their NAPLs when in the dissolved phase. However, this concept is not correct. Dissolved contaminants including dissolved PAHs move with the groundwater and do not "float" or "sink." Once a contaminant is dissolved, there is no density driver associated with contaminant movement. The behavior of dissolved phase contaminants has been discussed in many publications. For example, an API publication by Nichols and Roth (2006) makes the following observations: "Although LNAPLs may float and DNAPLs may sink when in pure phase, the constituents that dissolve from these free-phase mixtures into groundwater are neutrally buoyant." And "The factors that control groundwater flow direction (and therefore, any resulting plume dive) are largely independent of the solutes comprising a dissolved-phase plume." Thus, there is no need for three shallow monitoring wells with screens intersecting the groundwater table. These wells will not provide any additional data to capture COC results during the groundwater monitoring program. The requirement to span the water table is only valid for LNAPL which has not been detected at this site. Therefore, the monitoring wells planned for installation at this site are entirely appropriate for assessing the presence and concentration of dissolved-phase PAHs in groundwater.

Secondly, RSA surficial geology largely consists of a variable thickness of heavy clay soils (i.e., the overburden) overlying weathered limestone bedrock, which is the situation at RSA-271. Historical drilling activities have shown that the clay is not uniformly saturated.

For example, well 271-RS2707, which was drilled adjacent to the removed UST at the site illustrates the nature of the groundwater occurrences at the site (Attachment 1). Descriptions of the overburden soils indicate moist clay was present in the subsurface to a depth of 30 feet below ground surface (bgs) where "free water" was indicated on the log. The well was deepened and the well screen was set across the "free water" zone from 39.5 to 29.5 feet bgs, with sand pack to 26.5 bgs. The initial water level during development was 20 feet bgs and was gauged at 18.51 feet bgs on December 15, 2015. The approximate 10-foot rise in water level from where water was encountered to where it was measured in the well suggests semi-confined to confined conditions are present. In this case, setting a well screen from 25 to 15 feet bgs (straddling the water level in the well) would likely have resulted in setting a dry well.

In general, wells are set where free water is encountered in the subsurface as observed by the site geologist. Pre-selecting well screen intervals based on measured water levels does not take into account the wide variability in depth of first saturation that exists across RSA. As discussed for example, a groundwater monitoring well screened across the measured water level at location 271-RS2707 (i.e., 15-25 feet bgs) would have a very high probability of being dry as there is no free water in this portion of the subsurface. The aquifer is behaving as a semi-confined aquifer in this part of RSA and as such approach to MNA as designed in this CMIP is to complete the wells where groundwater is encountered in the subsurface.

However, to address the concern that LNAPL may be present at this site and has simply not been observed during the RFI, a contingency will be added to Chapter 5 of this CMIP in the event LNAPL or evidence of LNAPL is encountered such as elevated PID readings, observed odors or sheens. The contingency will note that an LNAPL investigation will be implanted using piezometers, and wells as needed. If LNAPL is confirmed, an additional supplemental remedy may be needed.

In summary, for the reasons stated above, no changes are recommended to the well installation planned in this CMIP. However, ADEM's two issues raised in this comment will be addressed by adding contingencies to the CMIP if unexpected conditions are encountered at this site in the future.

Nichols Eric M. and Tracy L. Roth (2006), *Downward Solute Plume Migration: Assessment, Significance, and Implications for Characterization and Monitoring of "Diving Plumes,"* American Petroleum Institute (API), API Soil and Groundwater Technical Task Force Bulletin 24, April.